



Full-Scale Accelerated Performance Testing for Superpave & Structural Validation

•
Outcomes - Ongoing Activities - Future APT Experiments



Hilton Head Island, South Carolina
November 9-12, 2009

FHWA Turner-Fairbank Highway Research Center
Office of Infrastructure R&D
Nelson Gibson, nelson.gibson@dot.gov



Background

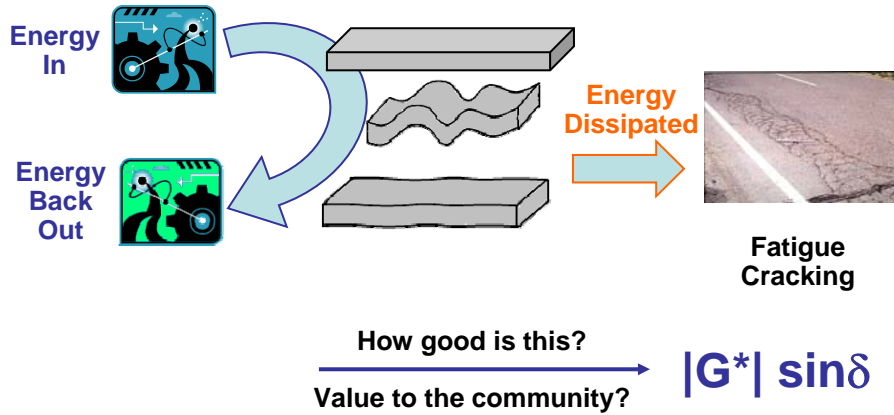
- Superpave Performance Grade
Purchase Specification

PG 64 17 -22

$|G^*| \sin \delta$



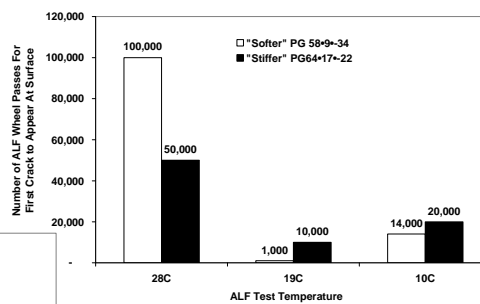
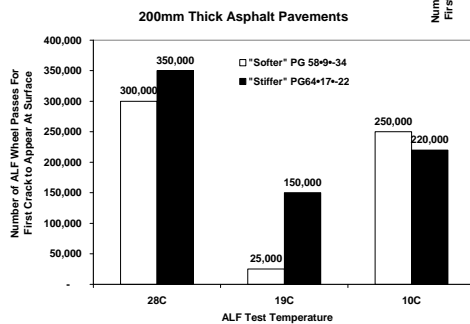
Background



Post-SHRP 1993 FHWA ALF

100mm Thick Asphalt Pavements

Better fatigue performance
 at cooler and warmer



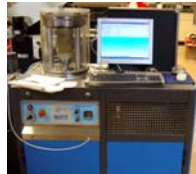
Stiffer binder performed
 better at intermediate
 temperature
 Not intent of specification

OBJECTIVES for *Full-Scale Accelerated Performance Testing for Superpave & Structural Validation*


- **Recommendations that provide AASHTO with a binder purchase specification that is “blind” to the type of modification.**



Secondary Objectives



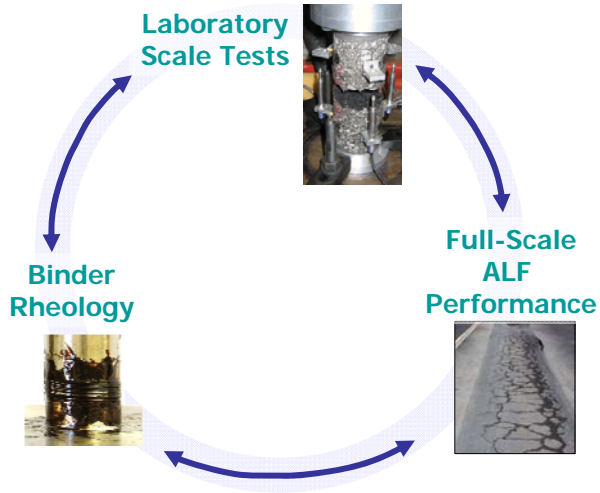
Candidate Binder Parameters

- **Stiffness Reduction**
 - Time Sweep 
 - Stress Sweep
 - Large Strain Time Sweep Surrogate
- **Fracture**
 - CTOD
Critical Tip Opening Displacement
- **Strength**
 - Binder Yield Energy

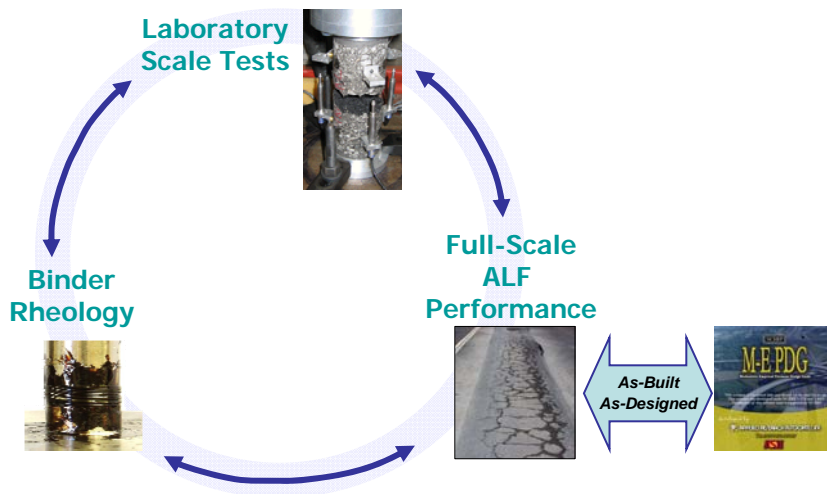




Approach to Evaluation



Approach to Evaluation



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Lane 1 Lane 2 Lane 3 Lane 4 Lane 5 Lane 6 Lane 7 Lane 8 Lane 9 Lane 10 Lane 11 Lane 12

100 mm Asphalt Pavement Layer

150 mm Asphalt Pavement Layer

CR-AZ	PG	Air-Blown	SBS LG	CR-TB	Elvaloy	Fiber	PG	SBS	Air-Blown	SBS LG	Elvaloy
PG70-22	70-22						70-22	64-40			
Removed 100 mm of Existing CAB						Removed 50 mm of Existing CAB					
100 mm of New No. 21A CAB Under All 12 Lanes →											
Existing VDOT No. 21A Crushed Aggregate Base (CAB) (25-mm Nominal Maximum Aggregate Size) Existing Depths of CAB are 560 mm under Lanes 1 and 2 And 460 mm under Lanes 3 to 12 Bottom of CAB to Pavement Surface is 660 mm											

- PG 70-22 = Unmodified Asphalt Binder Control
- CR-AZ = Crumb Rubber Asphalt Binder, Arizona DOT Wet Process
- CR-TB = Crumb Rubber Asphalt Binder, Terminal Blend
- Elvaloy = Ethylene Terpolymer Modified Asphalt Binder
- SBS LG = Styrene-Butadiene-Styrene Modified Asphalt Binder with Linear Grafting
- SBS 64-40 = Styrene-Butadiene-Styrene Modified Asphalt Binder Graded to be a PG 64-40
- Air-Blown = Air-Blown Asphalt Binder
- Fiber = Unmodified PG 70-22 Asphalt Binder with 0.2 Percent Polyester Fiber by Mass of the Aggregate.



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Binder Description	Control 70-22		Control 70-22		Control 70-22		Air Blown		Terpolymer		SBS-LG		SBS 64-40		CR-TB	
PTF Test Lane	1 (Bottom)		2		7 8		3 10		6 12		4 11		9		5	
Asphalt Thickness (mm)	50		100		100 150		100 150		100 150		100 150		150		100	
Performance Grade (PG)	70 -22		70 -22		70 -22		70 -28		70 -28		70 -28		70 -34		76 -28	
Continuous Performance Grade	72 -23		72 -23		72 -23		74 -28		74 -31		74 -28		71 -38		79 -28	
T(°C) when MVR(orig)=50@1.225kg	73.5		74.6		72.6		74.8		81.2		77.2		77		80.6	
T(°C) when G* /sinδ (ORIG) = 1kPa	73.2		72.8		72.1		75.5		78		75.1		71.7		79.5	
T(°C) when G* /sinδ (RTFOT) = 2.2kPa	72.3		72.9		73.2		74.1		74.5		74.4		71.8		81.4	
T(°C) when G* /sinδ (PAV) = 5MPa	26.7		25.4		26.1		22.6		14.3		17.7		8.6		17.9	
T(°C) when S(60) (PAV) = 300MPa	-13.5		-13.8		-13.5		-18.9		-21.3		-22.7		-28.5		-22.9	
T(°C) when m(60) (PAV) = 0.3	-13.3		-13.8		-13		-18.3		-24.1		-19.3		-29.5		-17.6	
Cracking T (°C) using BBR + DT	-20.3		-23.5		-21.8		-26.8		-33.1		-35.2		-41		-31.6	
Cracking T (°C) using BBR alone	-21.3		-22.2		-22.9		-27.1		-31.1		-33.7		-36		-32.9	



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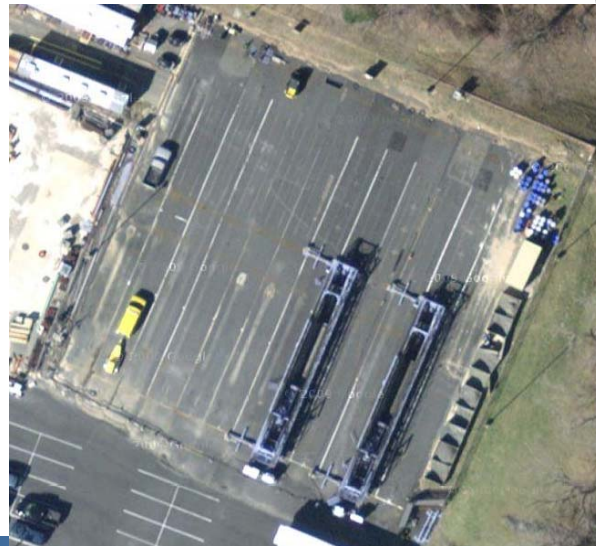
Binder Description	Control 70-22		Control 70-22		Control 70-22		Air Blown		Terpolymer		SBS-LG		SBS 64-40	CR-TB		
PTF Test Lane	1 (Bottom)		2		7	8	3	10	6	12	4	11	9	5		
Asphalt Thickness (mm)	50		100		100	150	100	150	100	150	100	150	150	100		
Performance Grade (PG)	70	-22	70	-22	70	-22	70	-28	70	-28	70	-28	70	-34	76	-28
Control Pave Formative Grade	72	-23	72	-23	73	-23	74	-28	74	-31	74	-28	71	-38	79	-31
T ₁₀₀ when MVN(mg)=500(1.25kg)	71.5		71.6		73.5		74.9		81.1		77.3		77		81.6	
T ₁₀₀ when T* sinδ (CRIO) = 1kPa	71.2		72.8		72.1		75.3		78		75.1		71.7		75.5	
T ₁₀₀ when T* sinδ (CRIO) = 2.5kPa	71.1		72.9		71.1		74.1		75.1		73.4		71.8		81.4	
T ₁₀₀ when G* ksinδ (PAV) = 5MPa	26.7		25.4		26.1		22.6		14.3		17.7		8.6		17.9	
T ₁₀₀ when BMO (PAV) = 30141Pa	-12.5		-12.3		-11.5		-13.9		-21.3		-22.7		-28.5		-22.1	
T ₁₀₀ when m _{0.01} (PAV) = 0.1	-13.4		-13.3		-11		-13.3		-24.1		-19.1		-28.5		-17.5	
Cracking T ₁₀₀ using BBR = 10T	-20.1		-21.5		-21.8		-23.8		-33.1		-35.7		-41		-31.1	
Cracking T ₁₀₀ using BBR 10ms	-21.3		-22.2		-22.9		-23.1		-31.1		-33.7		-36		-32.9	



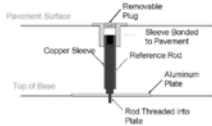
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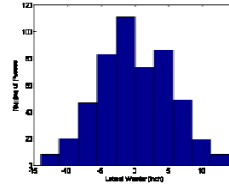
The Pavement Test Facility



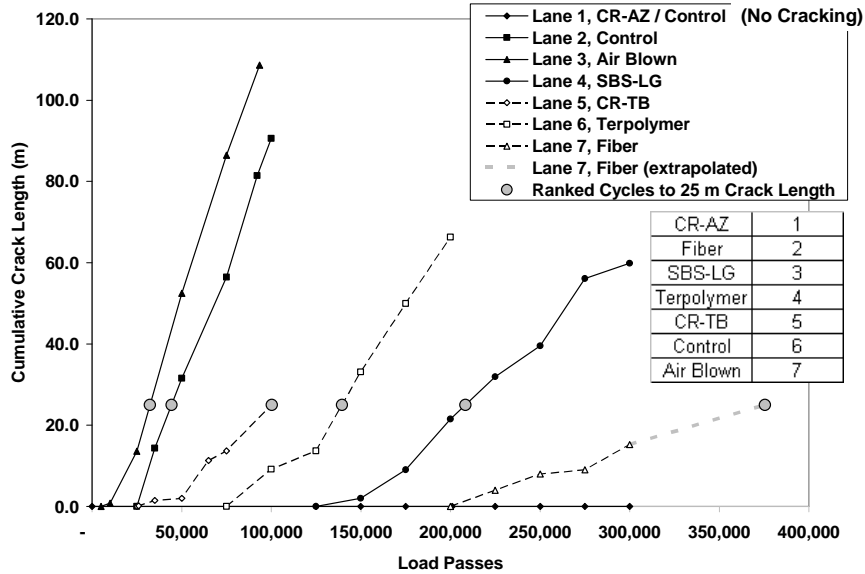
Loading Conditions

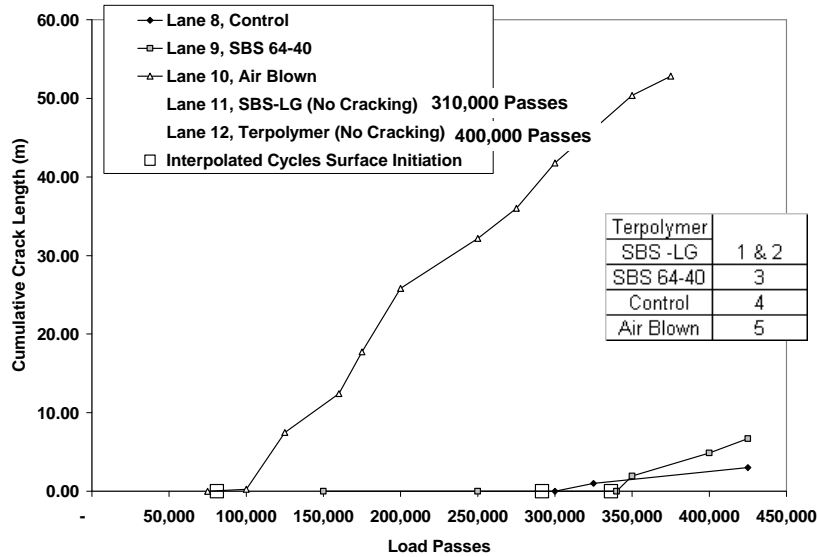


64°C (147°F)
100 psi Inflation
10,000 pounds
No Wander

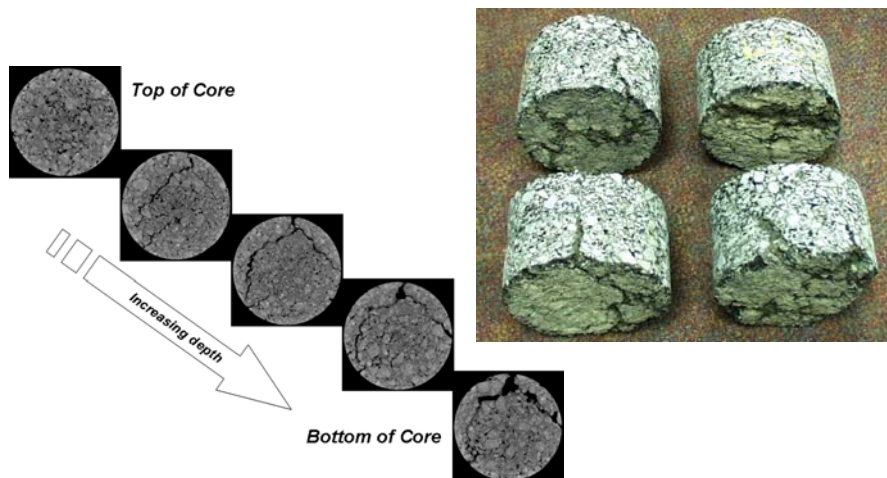


19°C (66°F)
120 psi Inflation
16,000 pounds
Wheel Wander





Confirmation of Bottom Up Cracking

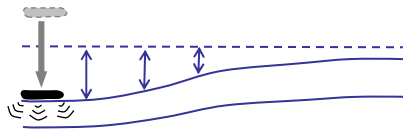




As-Built Consistency

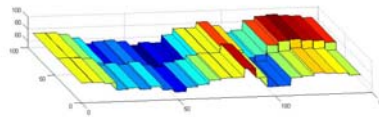


Falling Weight Deflectometer

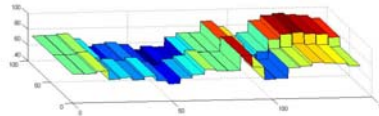


As-Built Consistency

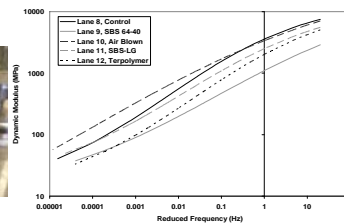
Crushed
Aggregate Base
Stiffness



A-4
Subgrade
Stiffness



HMA
Stiffness $|E^*|$



Laboratory Scale Fatigue Performance

Then...

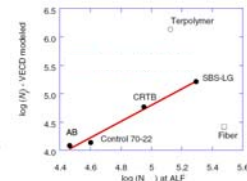
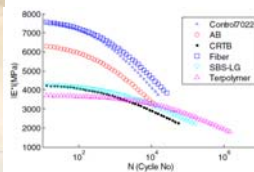


...Now

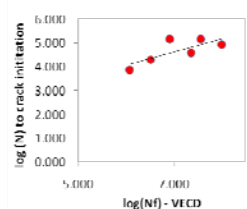
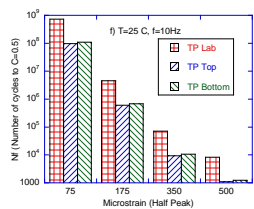


Laboratory Scale Fatigue Performance

Lab Fabricated



Field Compacted

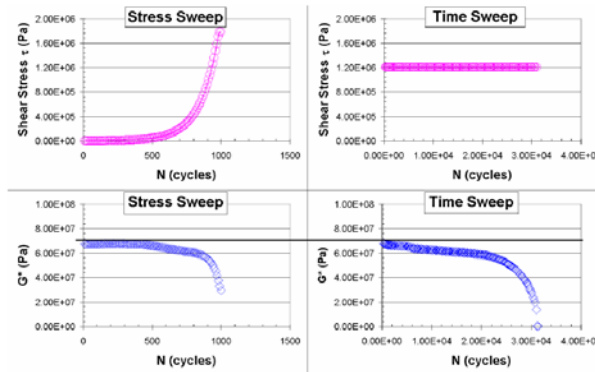




Time Sweep and Stress Sweep



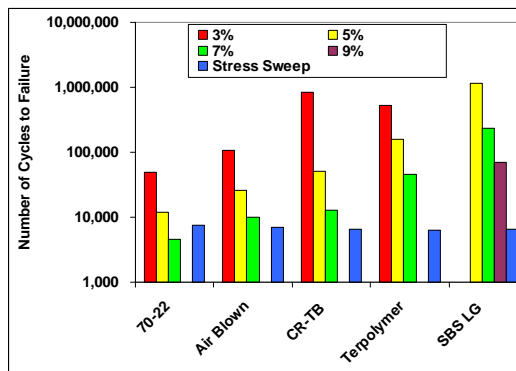
Martono, W. and H. Bahia, "Developing a Surrogate Test for Fatigue of Asphalt Binders," TRB 87th Annual Meeting Compendium of Papers DVD (2008)



Time Sweep and Stress Sweep



Martono, W. and H. Bahia, "Developing a Surrogate Test for Fatigue of Asphalt Binders," TRB 87th Annual Meeting Compendium of Papers DVD (2008)

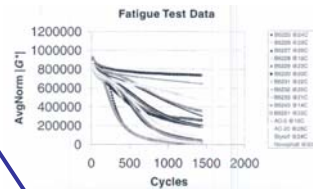


Large Strain Time Sweep Surrogate

Shenoy, A., (July 2002) "Fatigue Testing and Evaluation of Asphalt Binders Using the Dynamic Shear Rheometer," *ASTM Journal of Testing and Evaluation*, Vol. 30, No. 4, pp 303-312

$T=15^{\circ}\text{C}$ $\gamma = 0.1\%$
 $\gamma = 80\%$
 $T=20^{\circ}\text{C}$ $\gamma = 0.1\%$
 $\gamma = 80\%$
 $T=25^{\circ}\text{C}$ $\gamma = 0.1\%$
 $\gamma = 80\%$
 $T=30^{\circ}\text{C}$ $\gamma = 0.1\%$
 $\gamma = 80\%$

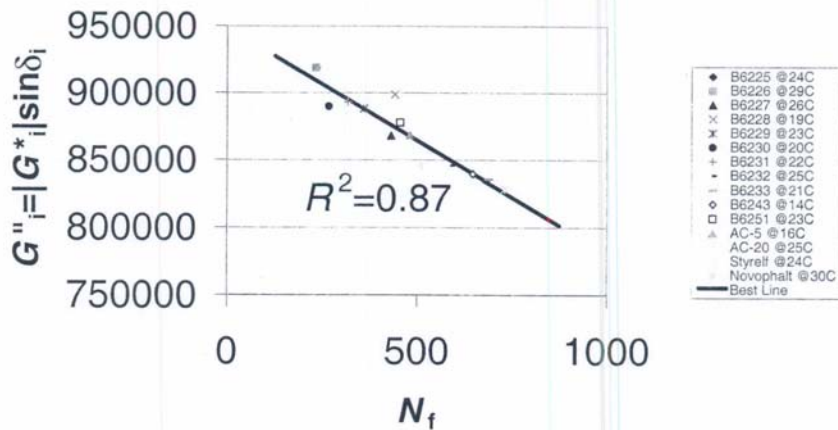
$T^{\circ}\text{C}$
 where
 $\gamma = 25\%$
 &
 $|G^*| = 1 \text{ MPa}$



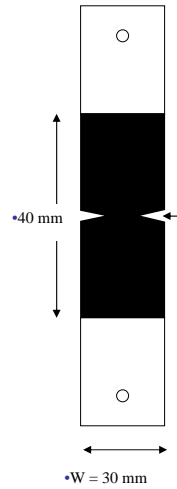
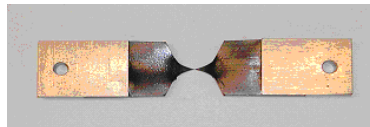
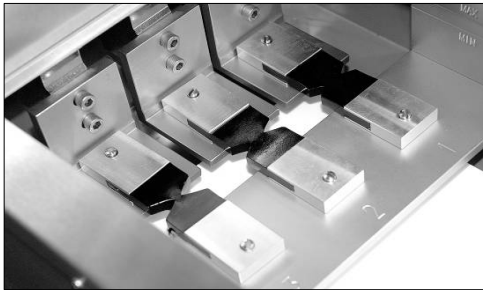
X62 larger than PG

Large Strain Time Sweep Surrogate

Initial G'' versus Cycles to Failure

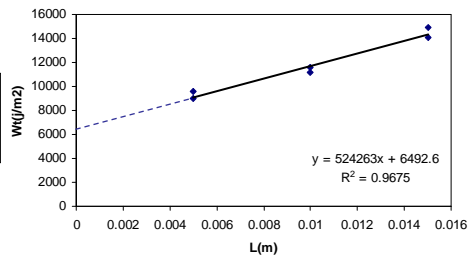
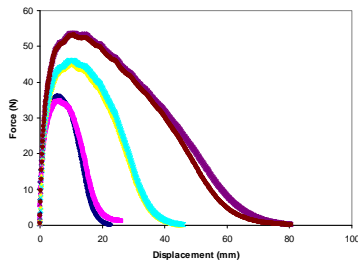


Critical Tip Opening Displacement



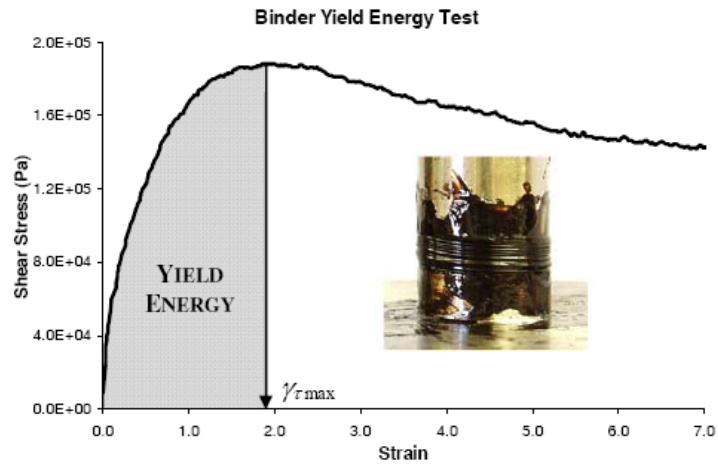
Critical Tip Opening Displacement

- CTOD is a measure of strain tolerance in the presence of a crack



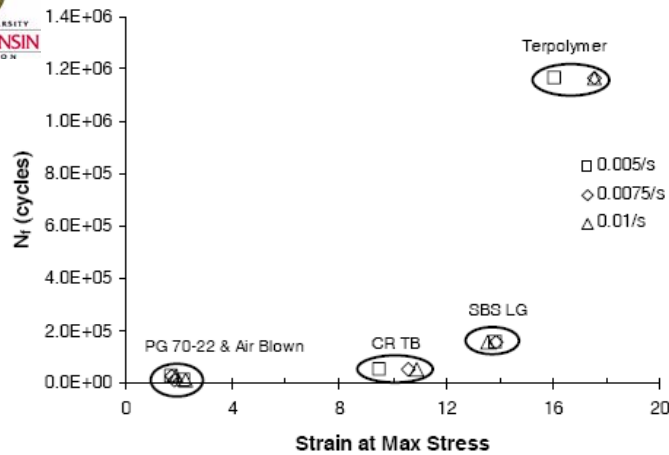


Binder Yield Energy Test

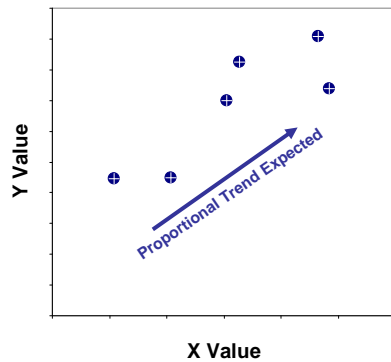


Binder Yield Energy Test

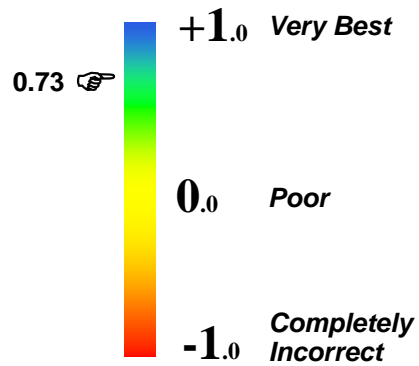
Strain at Max Stress vs. 5% Time Sweep N_f



How will Binder Parameter Candidates be Compared to Each Other?



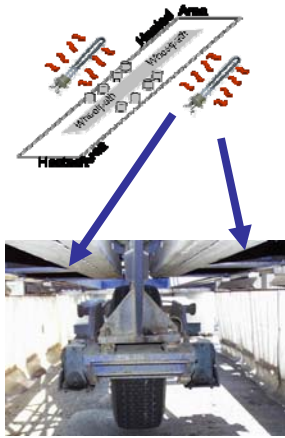
Rank Order Quality Indicator



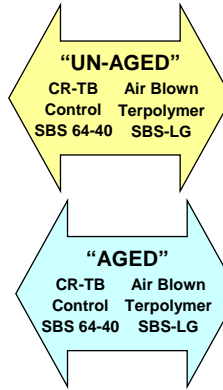
Binder Parameter	Rank Order Quality Indicator	
	Lab-Scale Fatigue Ranking	Full-Scale ALF Cracking
Superpave	-0.8 ↘	-0.6 ↘
Time Sweep	+0.6 ↗	+0.8 ↗
Stress Sweep	-0.6 ↗ ✘	-0.4 ↗ ✘
Large-Scale Time Sweep Surrogate	-0.6 ↘	-0.4 ↘
CTOD	+0.8 ↗	+1.0 ↗
BYET	+1.0 ↗	+0.8 ↗

Ongoing: Accelerated Aging

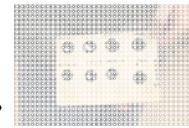
- Objective: Double (X2) the number of data points



Full Scale Fatigue Cracking

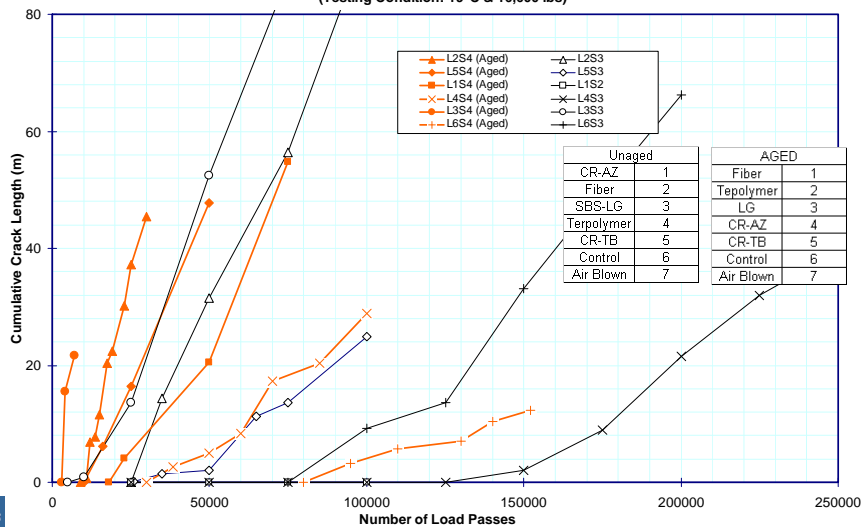


Binder Parameters



Ongoing: Accelerated Aging

Crack Length vs ALF Wheel Load Passes
(Testing Condition: 19°C & 16,600 lbs)



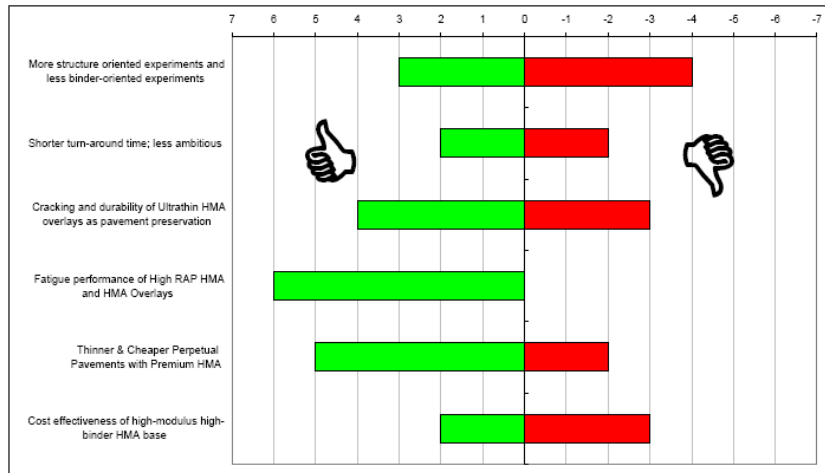
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	Thumbs Up	Thumbs Down	#1 Rank	#2 Rank	#3 Rank	#4 Rank	#5 Rank	#6 Rank	Average Score
More structure oriented experiments and less binder-oriented experiments	3	-4	0	2	3	3	2	5	4.3
Shorter turn-around time; less ambitious	2	-2	0	1	3	1	5	5	4.7
Cracking and durability of Ultrathin HMA overlays as pavement preservation	4	-3	4	3	2	2	3	2	3.2
Fatigue performance of High RAP HMA and HMA Overlays	6	0	5	7	1	3	1	0	2.3
Thinner & Cheaper Perpetual Pavements with Premium HMA	5	-2	6	3	5	2	1	0	2.4
Cost effectiveness of high-modulus high-binder HMA base	2	-3	2	1	2	4	3	3	3.9



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Conclusions

- Similarly PG graded materials exhibit different fatigue cracking performance
- MEPDG evaluated construction variation and it is small
- Composite structure with wet process crumb rubber has the ability to arrest or slow propagating cracks
- Fibers are very effective in reducing fatigue cracking
- Aging has significant influence on ranked order of performance



Conclusions

- There are better properties than $|G^*| \sin \delta$
 - CTOD and BYET appear to be among the most promising
 - ONGOING: 32 Binders From LTPP Test Sections



Asphalt Research Consortium

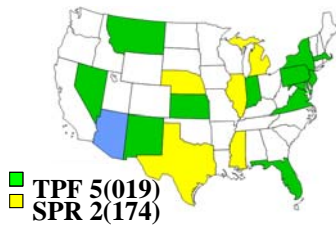
Western Research

- Binder tests cannot easily provide insight for structural effects (100 mm vs. 150 mm thick), fiber modified mixtures and composite pavements with CR-AZ
 - Stresses the importance of testing mix
 - Simplified fatigue testing in AMPT is of high value



Acknowledgements

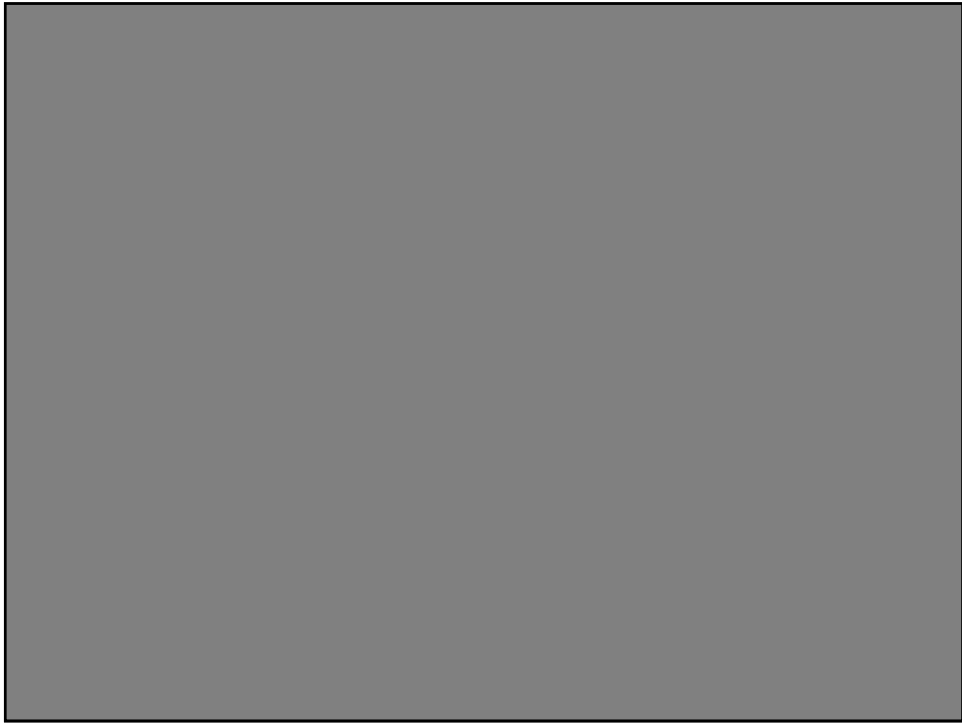
State DOT & Industry



- Citgo
- Dow
- Dupont
- Koch
- Paramount
- TexPar
- Trifinery, GCA
- Trumbull
- Wright Asphalt
- Martin Color Fi
- Bit Mat
- Mathy Construction
- Hot Mix Industries
- FNF Construction
- Consulpav
- Rubber Producers Assoc.
- ISS
- RTG
- NAPA



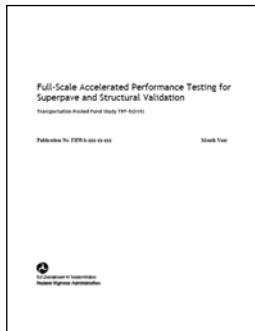
Nelson.Gibson@dot.gov



**Database
Application
(portable)
has been
developed**



**250-page
Detailed
Report**



TURNER-FAIRBANK HIGHWAY RESEARCH CENTER

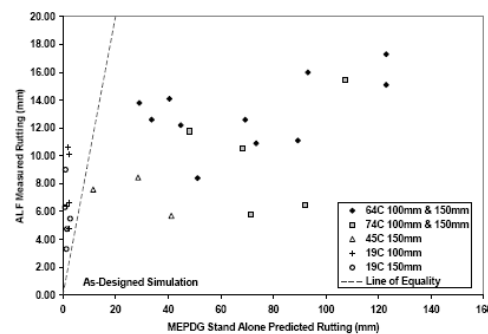
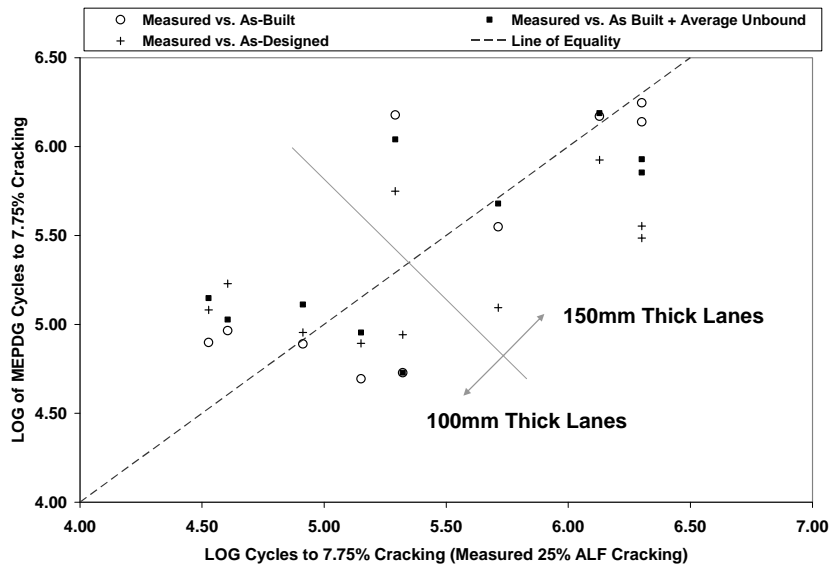
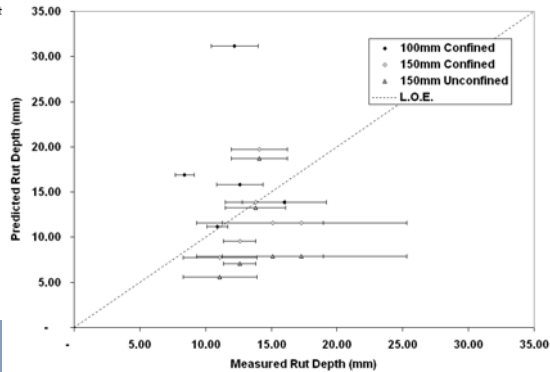
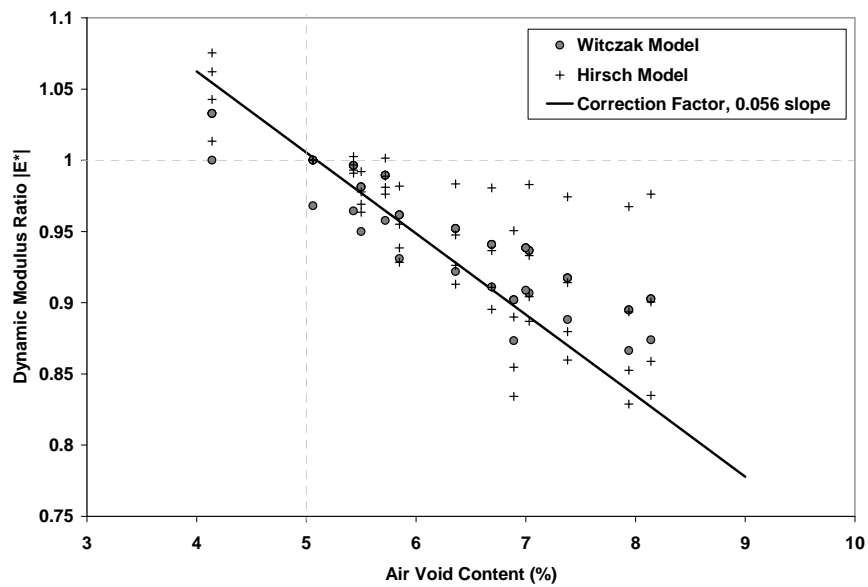


Figure 67. Measured and As-Designed predicted rutt





Abraham Maslow's *Law of the Instrument*



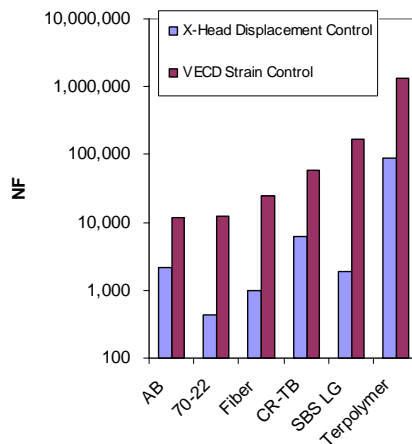
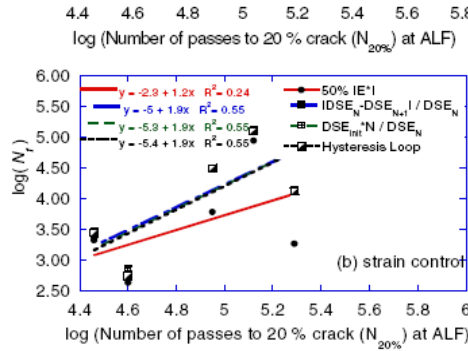
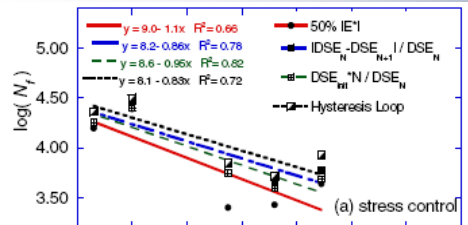


Binder Yield Energy Test



Johnson, C.M., H. Bahia and H. Wen, "Practical Application of Viscoelastic Continuum Damage Theory to Asphalt Binder Fatigue Characterization," *Asphalt Paving Technology*, Journal of the Association of Asphalt Paving Technologists (forthcoming).

"...based on the idea that [in HMA mixtures] there is an energy threshold where a material's resistance to deformation and resistance to damage are both overcome, leading to the propagation of cracking."





Summary

Binder Parameter	Regression Significance (1 - p-value)	
	Lab-Scale Fatigue Ranking	Full-Scale ALF Cracking
Superpave	67%	79%
Time Sweep	37%	98%
Stress Sweep	67%	86%
Large-Scale Time Sweep Surrogate	66%	80%
CTOD	43%	99%
BYET	72%	92%



Summary

Binder Parameter	Correlation Coefficient, R	
	Lab-Scale Fatigue Ranking	Full-Scale ALF Cracking
Superpave	-0.56	-0.68
Time Sweep	+0.29	+0.94
Stress Sweep	-0.75	-0.55
Large-Scale Time Sweep Surrogate	-0.55	-0.69
CTOD	+0.34	+0.95
BYET	+0.68	+0.83

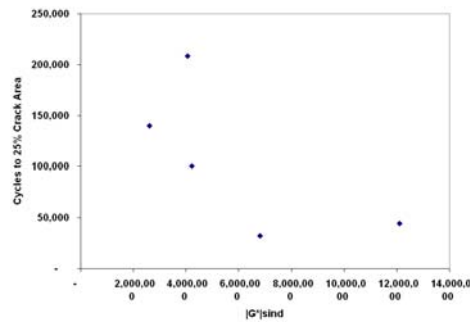
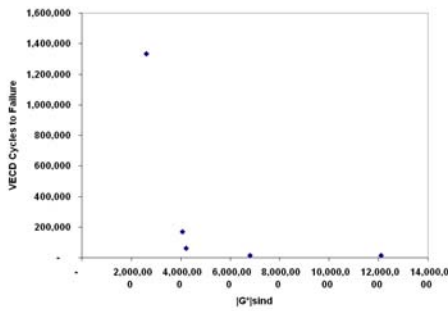


Summary

Binder Parameter	Coefficient of Determination, R ²	
	Lab-Scale Fatigue Ranking	Full-Scale ALF Cracking
Superpave	0.31	0.46
Time Sweep	0.09	0.88
Stress Sweep	0.31	0.57
Large-Scale Time Sweep Surrogate	0.30	0.48
CTOD	0.12	0.92
BYET	0.46	0.69

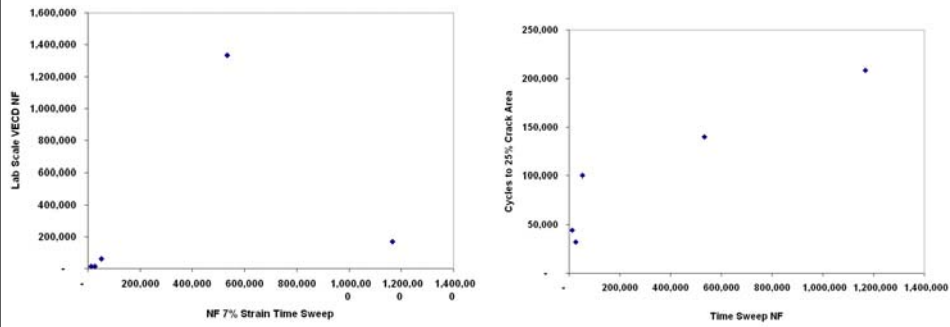


Scatter Plot - Superpave

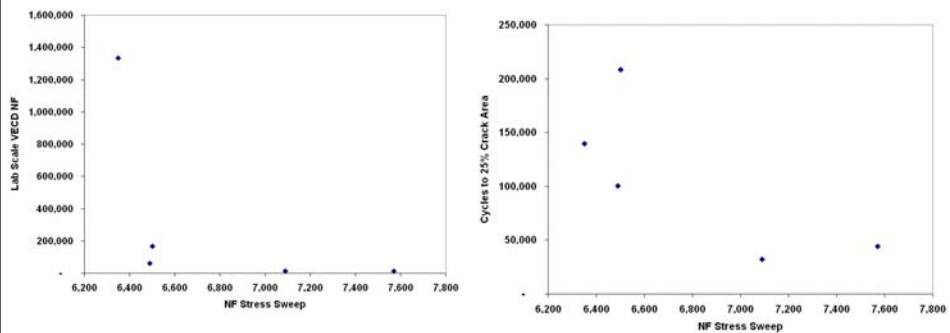




Scatter Plot – Time Sweep

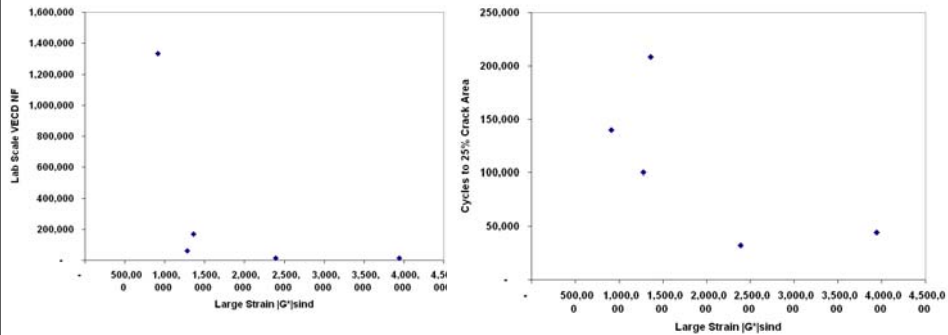


Scatter Plot – Stress Sweep

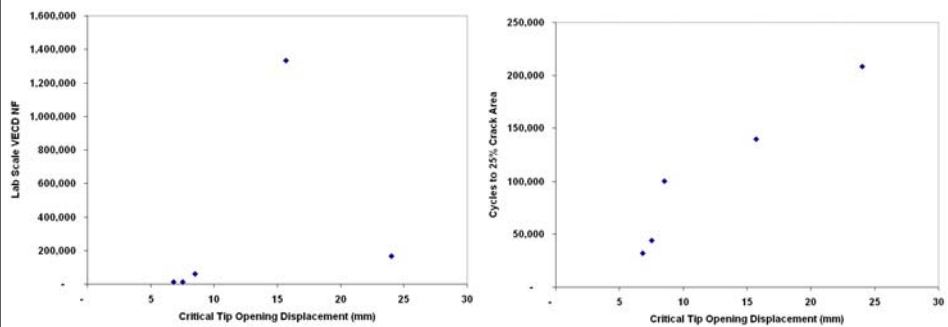




Scatter Plot – Large Strain Time Sweep Surrogate

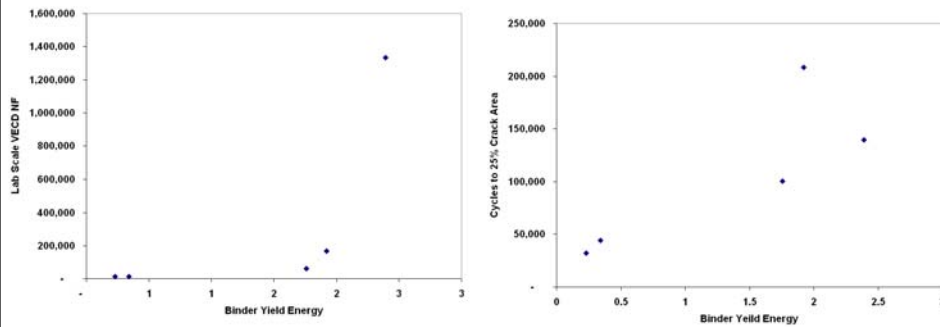


Scatter Plot – CTOD

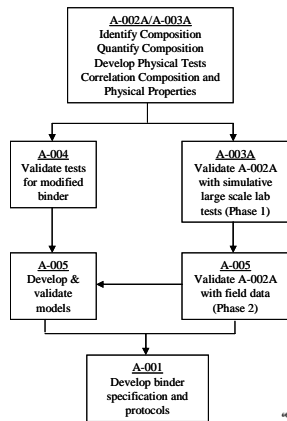




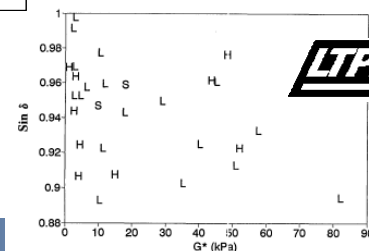
Scatter Plot – BYET



Historical Perspectives: SHRP



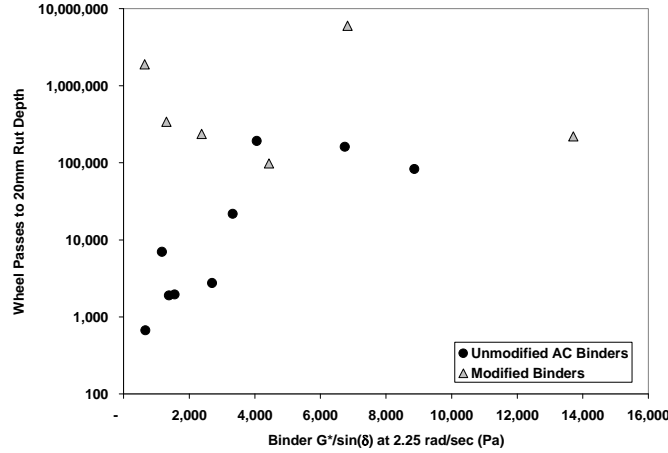
- Researchers were well aware of complexities and advanced tests & theories
- Introduction of DSR radically changed direction of research
- Linked dissipated energy to binder beam fatigue and Zacca-Wigmore test road
- Linked stress relaxation to “viscous component of stiffness” to wheel tracking



- Field validation from early LTPP test sections showed no binder specification by itself can explain field performance

Historical Perspectives: Post-SHRP

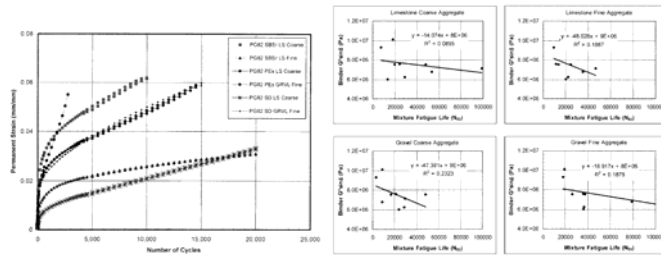
- FHWA ALF**



Historical Perspectives: Post-SHRP

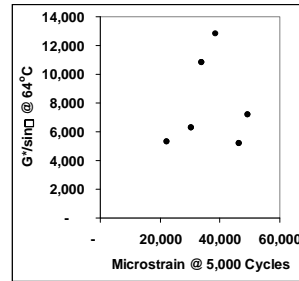
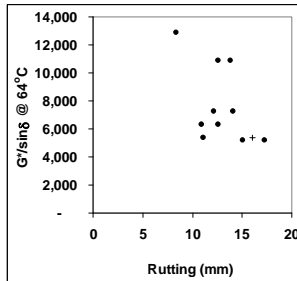
- NCHRP 9-10**

- HMA fatigue did not correlate to $|G^*|\sin\delta$
 - Binder fatigue time-sweep identified as potential candidate
- HMA permanent deformation did not correlate to $|G^*|\sin\delta$
 - Binder creep and recovery identified as potential candidate





Rutting: $|G^*|/\sin\delta$



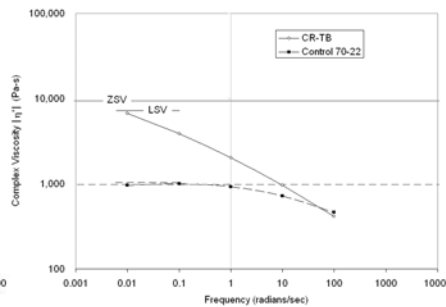
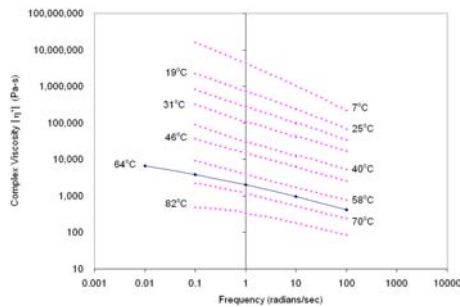
	Slope	Regression Significance (1-pvalue)	Kendall's tau	Kendall's tau Significance	R	R ²
$ G^* /\sin\delta$ vs. Rutting	- Correct	92%	- 0.31	98%	- 0.55	0.30
$ G^* /\sin\delta$ vs. Microstrain at 5,000 Cycles	+ Incorrect	11.2%	+ 0.2	64%	+0.078	0.006



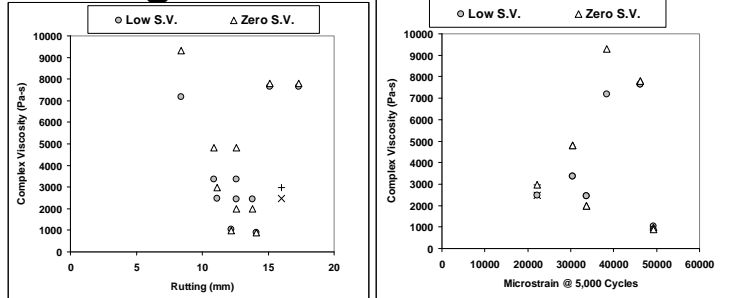
Rutting: Zero- & Low-Shear Viscosity

- Calculated from complex viscosity

$$\begin{aligned}
 \text{ZSV} \quad \eta_0^* &= \lim_{\omega \rightarrow 0} \frac{G''}{\omega} = \lim_{\omega \rightarrow 0} \frac{|G^*| \times \sin \delta}{\omega} & \& \quad \eta_L \Big|_{\omega=0.01 \text{ rad/sec}} = \frac{|G^*| \times \sin \delta}{\omega} \\
 \text{LSV}
 \end{aligned}$$



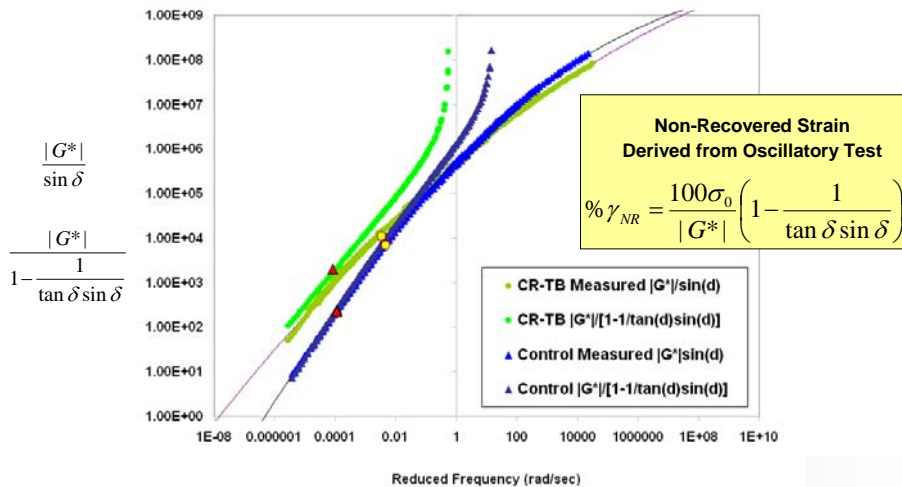
Rutting: Zero- & Low-Shear Viscosity



	Slope	Regression Significance (1-pvalue)	Kendall's tau	Kendall's tau Significance	R	R ²
LSV vs. Rutting	+ Wrong direction	26%	+ 0.09 Wrong direct	56%	+ 0.11	0.01
ZSV vs. Rutting	-	17%	+ 0.02	68%	- 0.07	0.005
LSV vs. $\mu\epsilon$ @ 5k Cycles	0	0.04%	- 0.14	61%	-0.0003	0
ZSV vs. $\mu\epsilon$ @ 5k Cycles	0	0.04%	- 0.14	61%	-0.0003	0

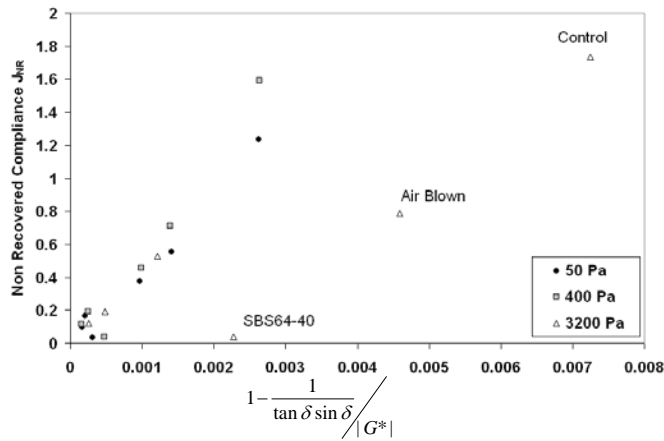
Rutting: Shenoy's Estimate for Non-Recovered Strain

Shenoy, Aron (2001). "Refinement of the Superpave specification parameter for performance grading of asphalt." *J. Transportation Engineering*, 127(5), 357-362.

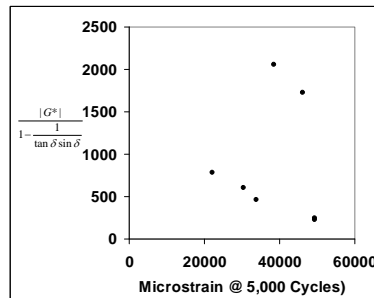
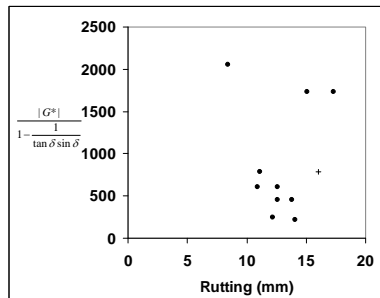


Rutting: Shenoy's Estimate for Non-Recovered Strain

$$\text{parameter: } \frac{|G^*|}{1 - \frac{1}{\tan \delta \sin \delta}} \quad J_{NR} \text{ estimate} = \frac{1 - \frac{1}{\tan \delta \sin \delta}}{|G^*|}$$



Rutting: Shenoy's Estimate for Non-Recovered Strain



	Slope	Regression Significance (1-pvalue)	Kendall's tau	Kendall's tau Significance	R	R ²
$\frac{ G^* }{1 - \frac{1}{\tan \delta \sin \delta}}$ vs. Rutting	+	48%	+ 0.23 Wrong direction	68%	+ 0.22 Wrong direction	0.05
$\frac{ G^* }{1 - \frac{1}{\tan \delta \sin \delta}}$ vs. $\mu\epsilon$ @ 5k Cycles	-	6.3%	- 0.33	81%	- 0.037	0.001



Non-Recovered Compliance J_{NR}

$$E = \frac{\sigma}{\varepsilon}$$

$$G = \frac{\tau}{\gamma}$$

$$D(t) = \frac{\varepsilon(t)}{\sigma_0}$$

$$J(t) = \frac{\gamma(t)}{\tau_0}$$

