

Ohio Accelerated Pavement Loading Facility

Ohio Accelerated Pavement Loading Facility

- Owner/Operator: Ohio University
- Location: Lancaster, Ohio, USA
- Constructed 1996-1997
- Enclosed Chamber with Controlled Loading, Air Temperature and Subgrade Moisture
- -10°F to 140° F
- Test Pit: (41 ft x 35 ft x 15 ft) 13.7 m x 11.6 m x 2.4 m deep

Ohio Accelerated Pavement Loading Facility



Ohio Accelerated Pavement Loading Facility



Paving at APLF



Sensor Installation



Performance of Shallow Cover Thermoplastic Pipes Subject to Temperature Change

Billy Jones

Presentation Overview

- Facility and Project Information
- Project Construction
- Instrumentation & Sample Data
- Summary



Ohio University APLF Facility

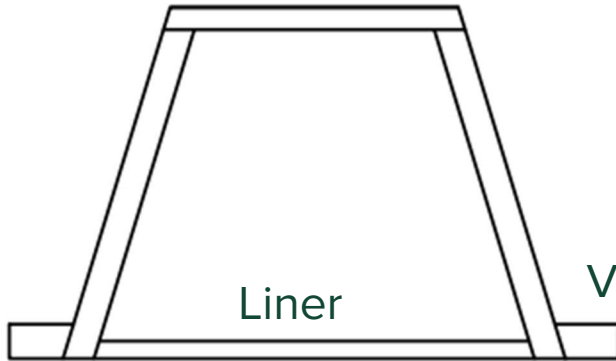
Shallow Cover Experiment

- Tests were done in Accelerate Pavement Loading Facility (APLF)
 - Temperature controlled
 - Fitted with Accelerated Pavement Testing Machine (APTM)
- APTM capable of exerting 30,000 lb dual tire load
- 3 high density polyethylene (HDPE), and 2 polypropylene (PP) pipes buried in testing pit
- 48-inch diameter pipes – Ohio DOT Item 304
- 36-inch diameter pipes – #57 Gravel
- All pipes subject to same cover conditions

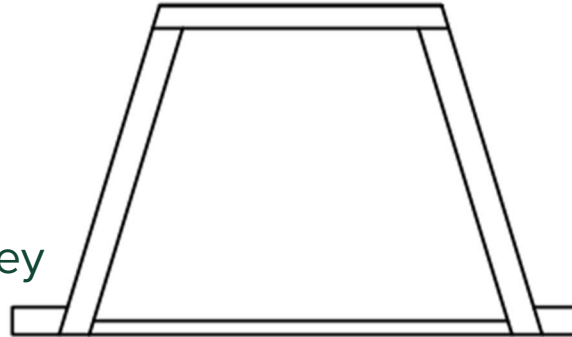


Pipe Material Information

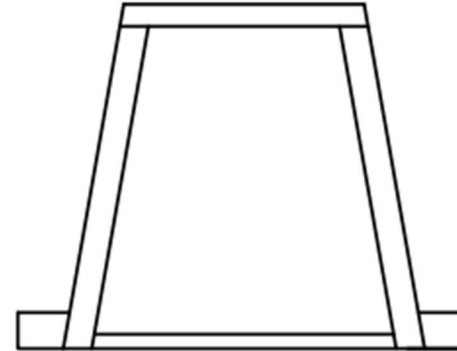
PP - 36"



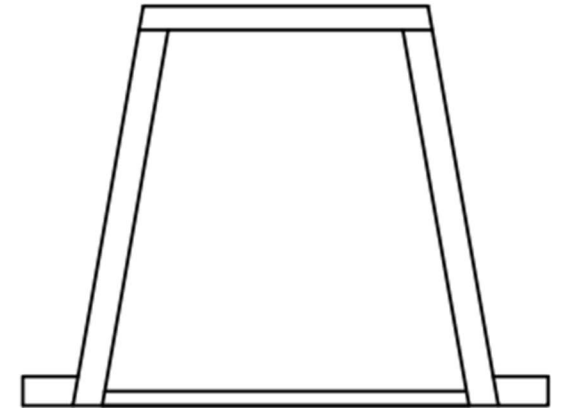
HDPE - 36"



PP - 42"



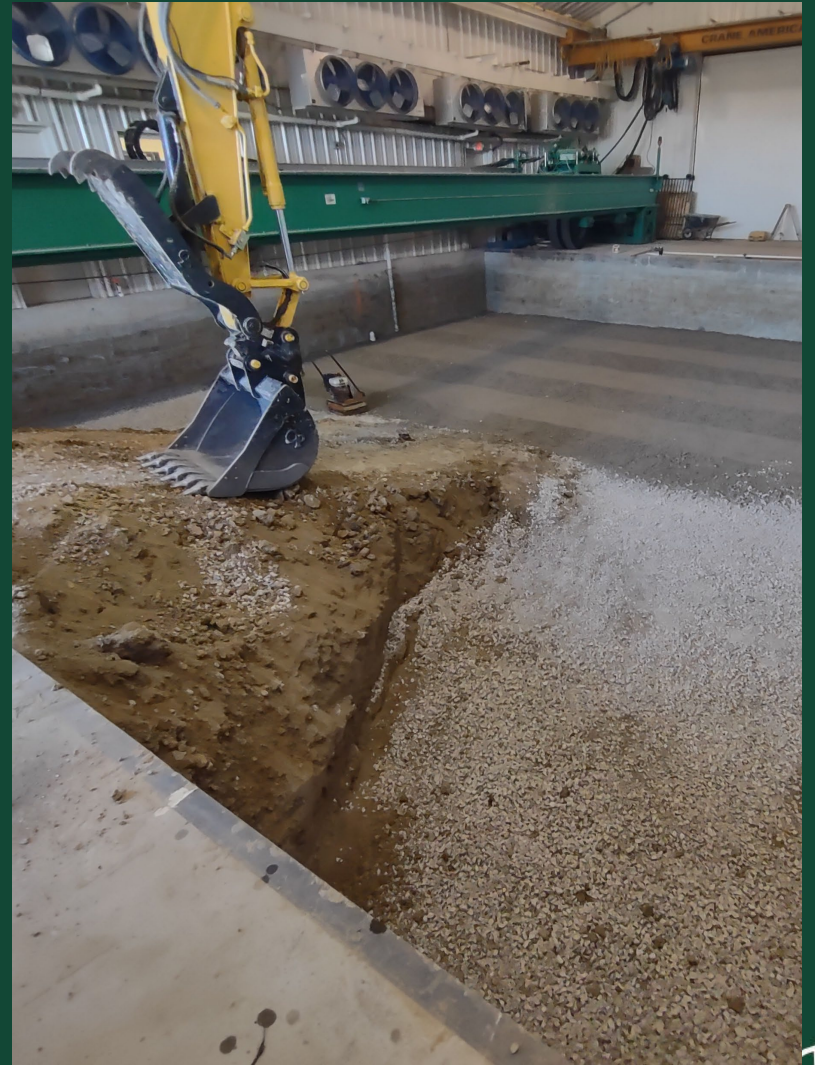
HDPE - 42"



Pipe Material	Young's Modulus (Short-term)	Pipe Material	Nominal Diameter (in)	Effective Area (in ²)	MOI (in ⁴)	Modulus (Long-term)	Ultimate Stress Limit (Long-term)
PP	175000	PP	36	0.302	0.3703	1000	1000
			42	0.34	0.3786		
HDPE	110000	HDPE	36	0.268	0.3299	900	900
			42	0.28	0.5295		

OHIO
UNIVERSITY

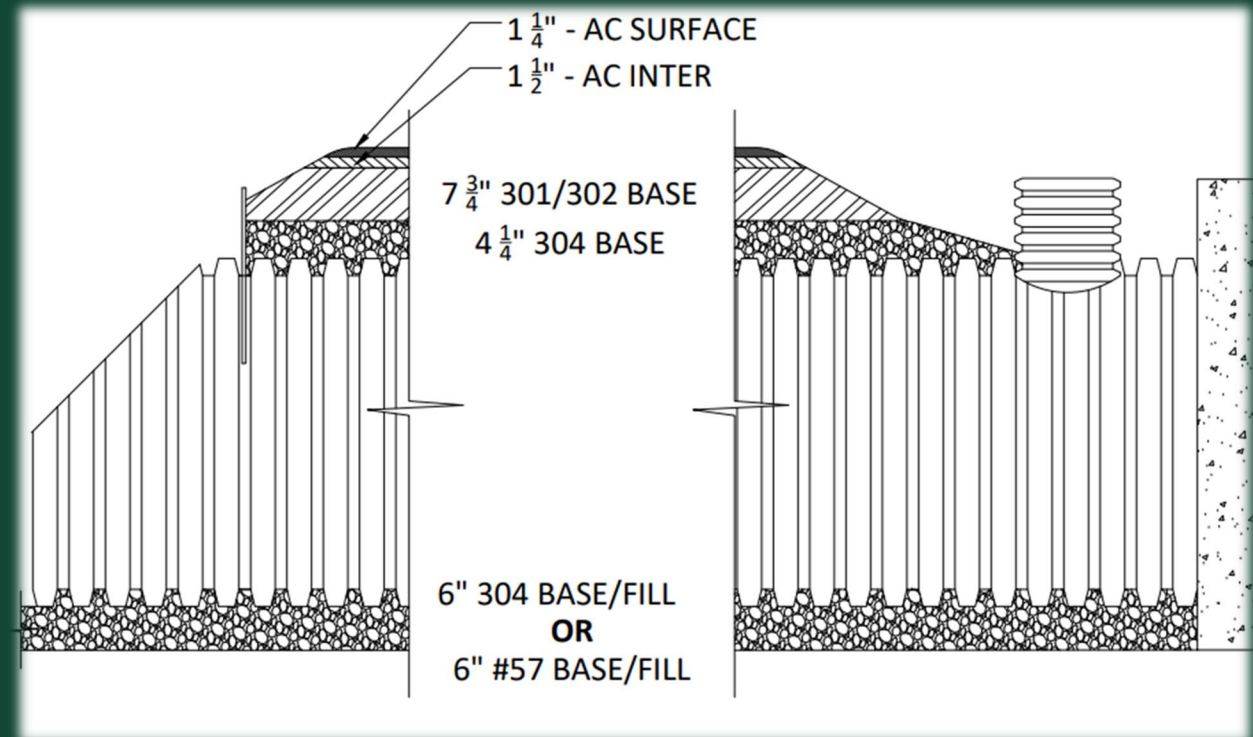
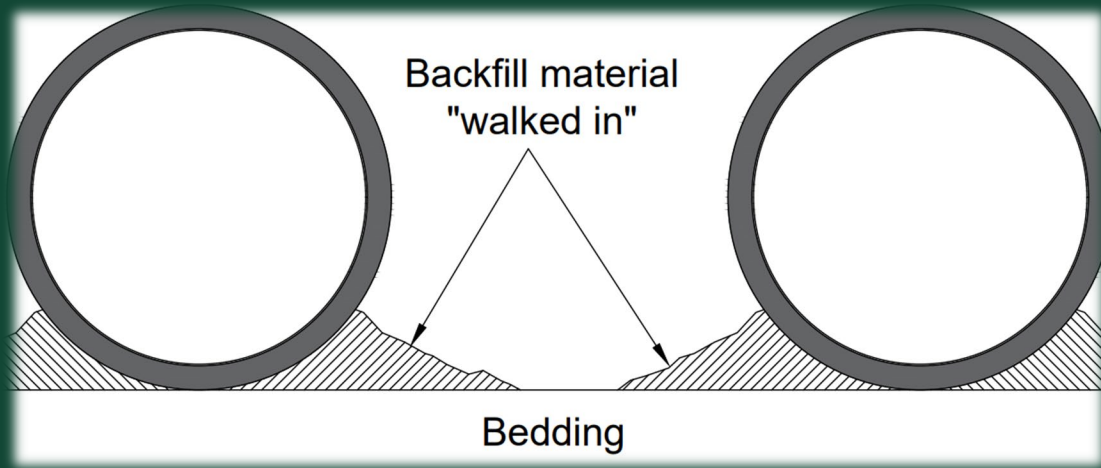
Pre-Construction Demolition





Embedment Construction Details

- ASTM F1668 Construction Procedures for Buried Plastic Pipe
- 36" - uncompacted #57 gravel
- 48" – 304, compacted 4-inch lifts
- 4 ¼" layer of compacted 304 overtop



Embedment Materials - #57 vs 304



ODOT Item 304



AASHTO #57 Gravel

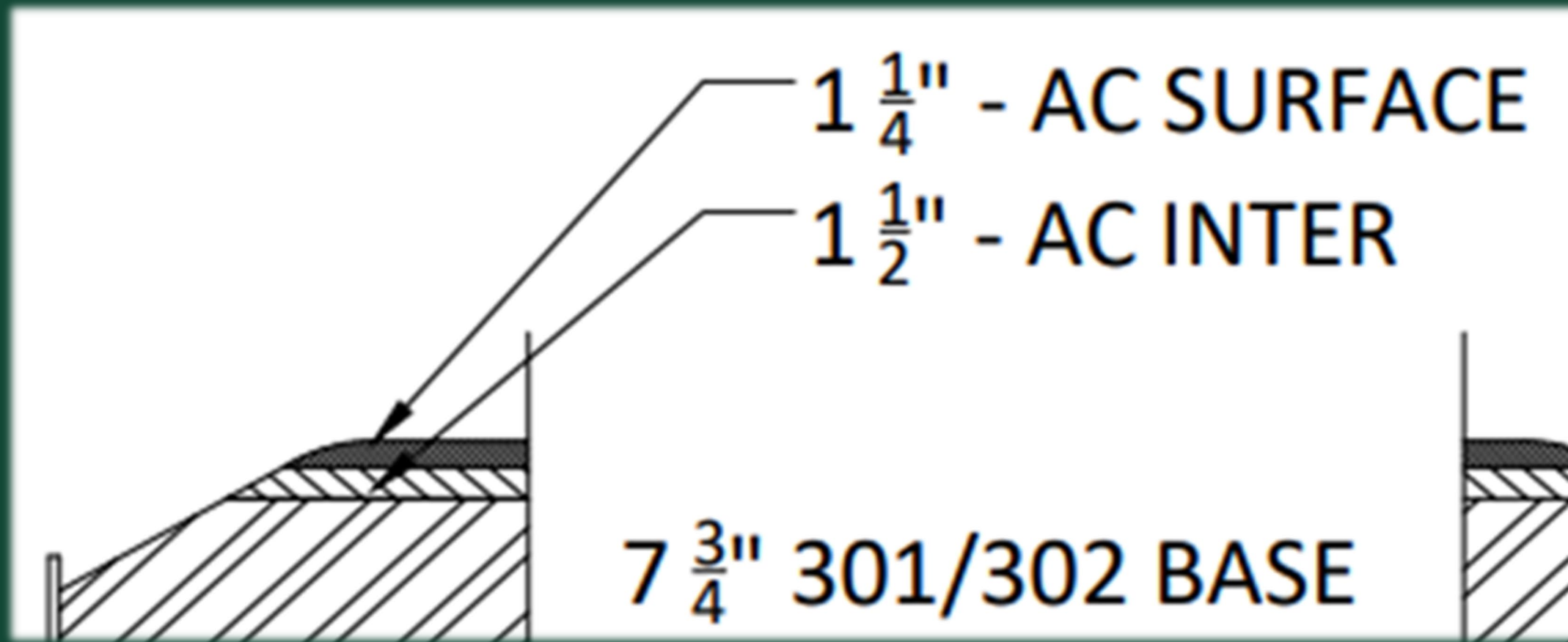
Embedment Material Soil Testing



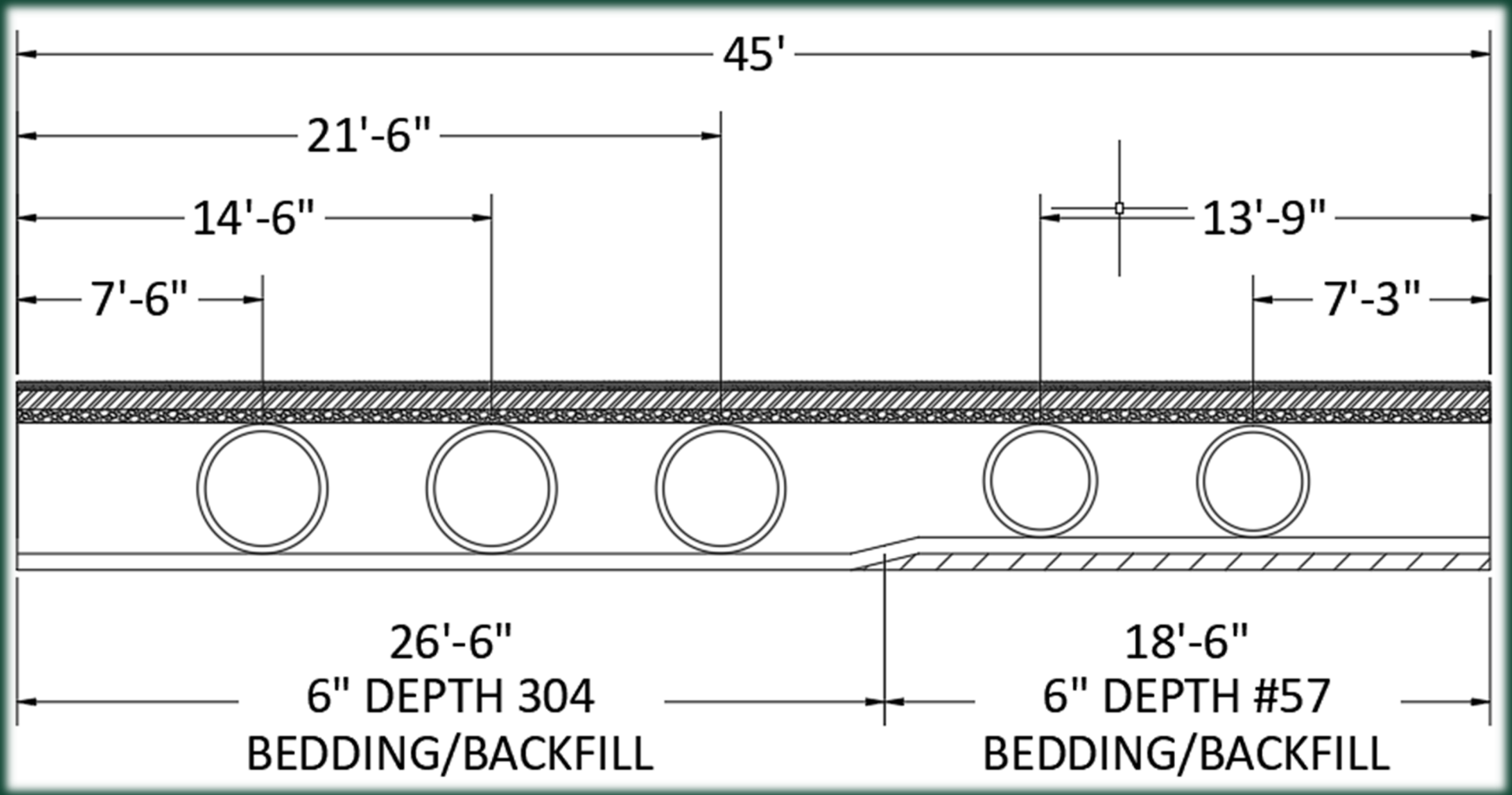
Embedment Construction



Asphalt Construction Details



Construction Details – Plan View





Instrumentation and Data Collection

Data Collection Intervals

ROOM
PIPE TEMP - (60-75F)
AC TEMP - (65-85F)

COLD
PIPE TEMP - (<60 F)
AC TEMP - (<65 F)

WARM/HOT
PIPE TEMP - (>75F)
AC TEMP - (>85 F)

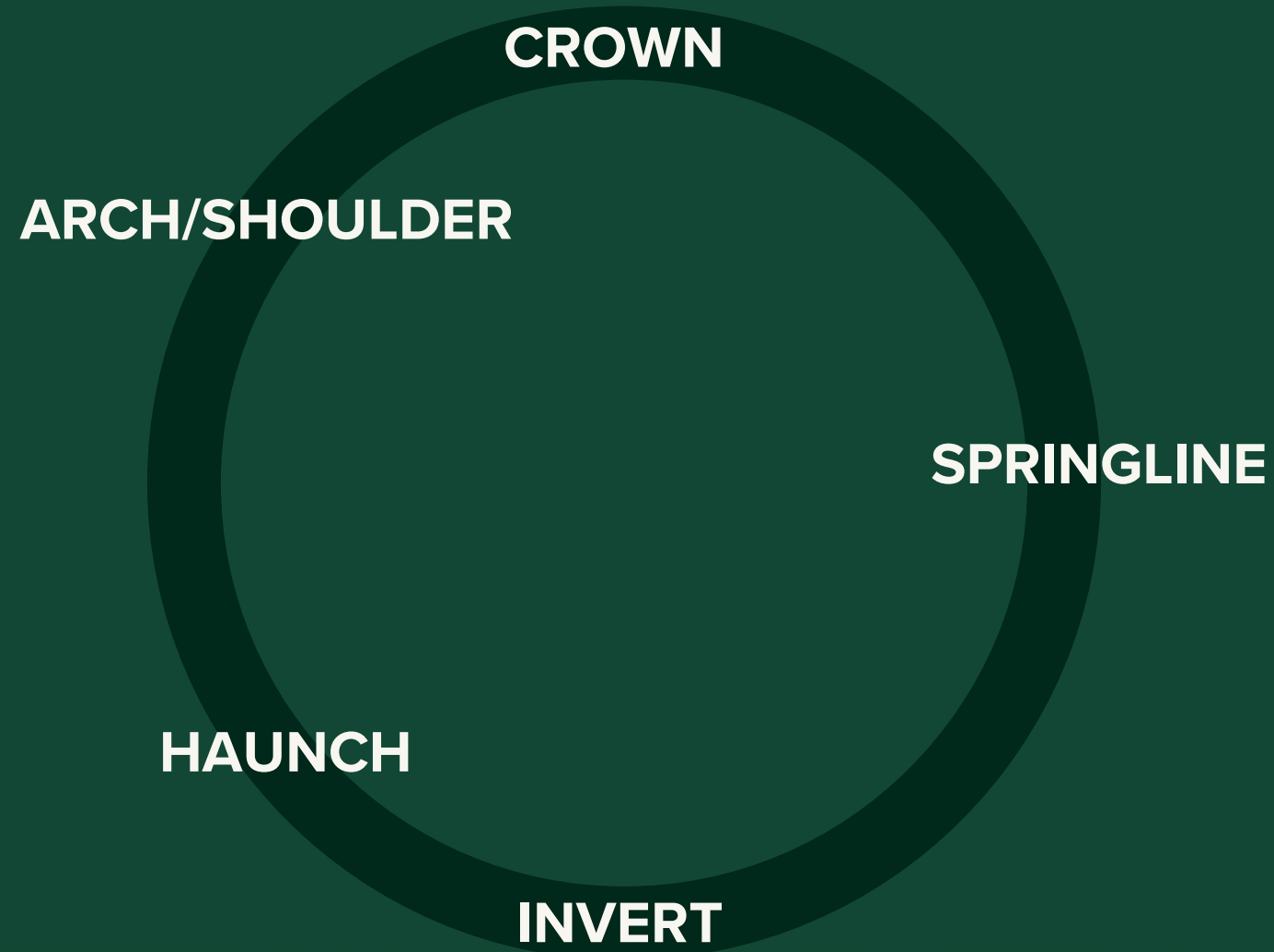
0-10K

10K-30K

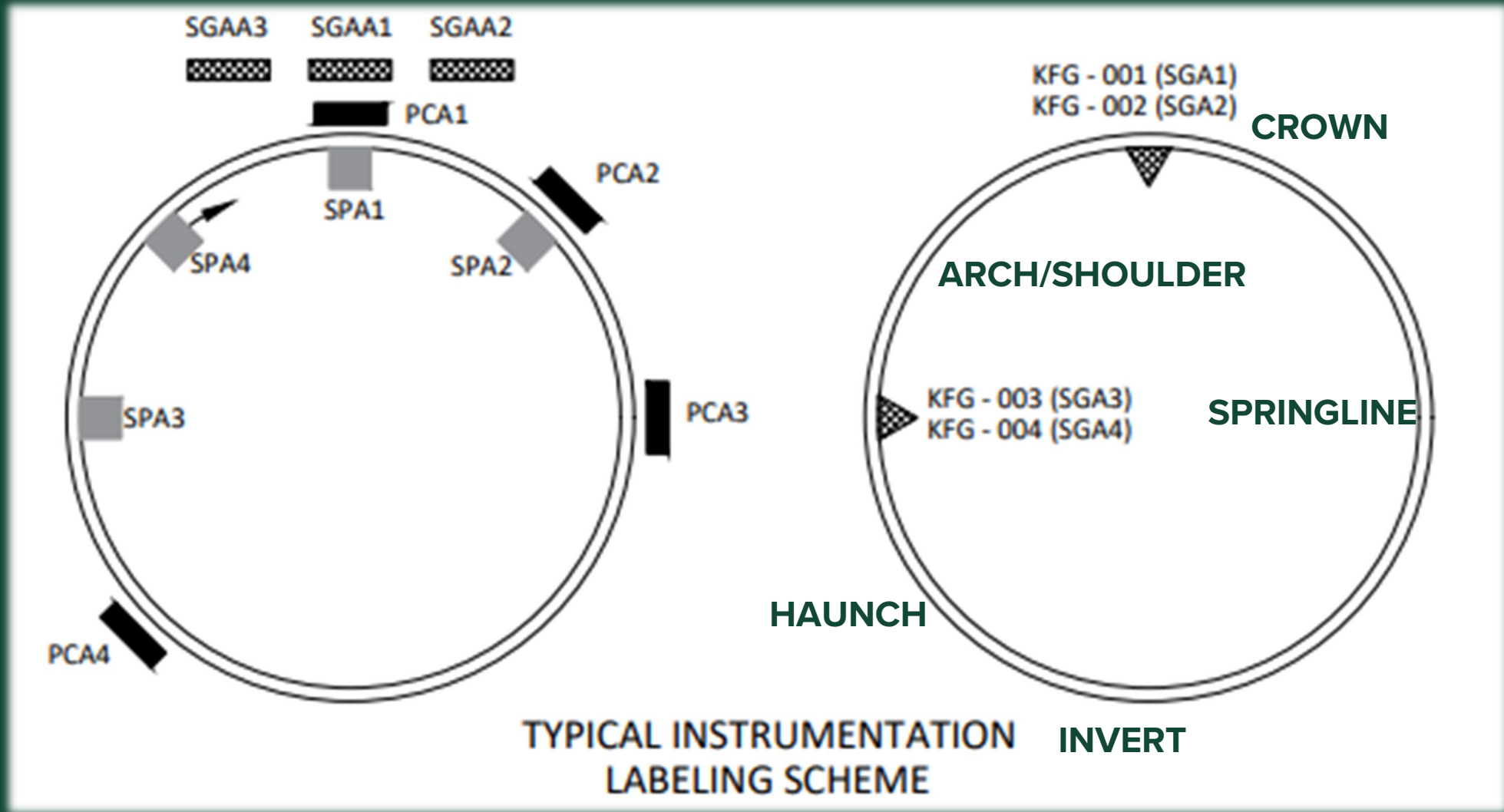
30K-
50K

50K-
80K

Pipe Nomenclature

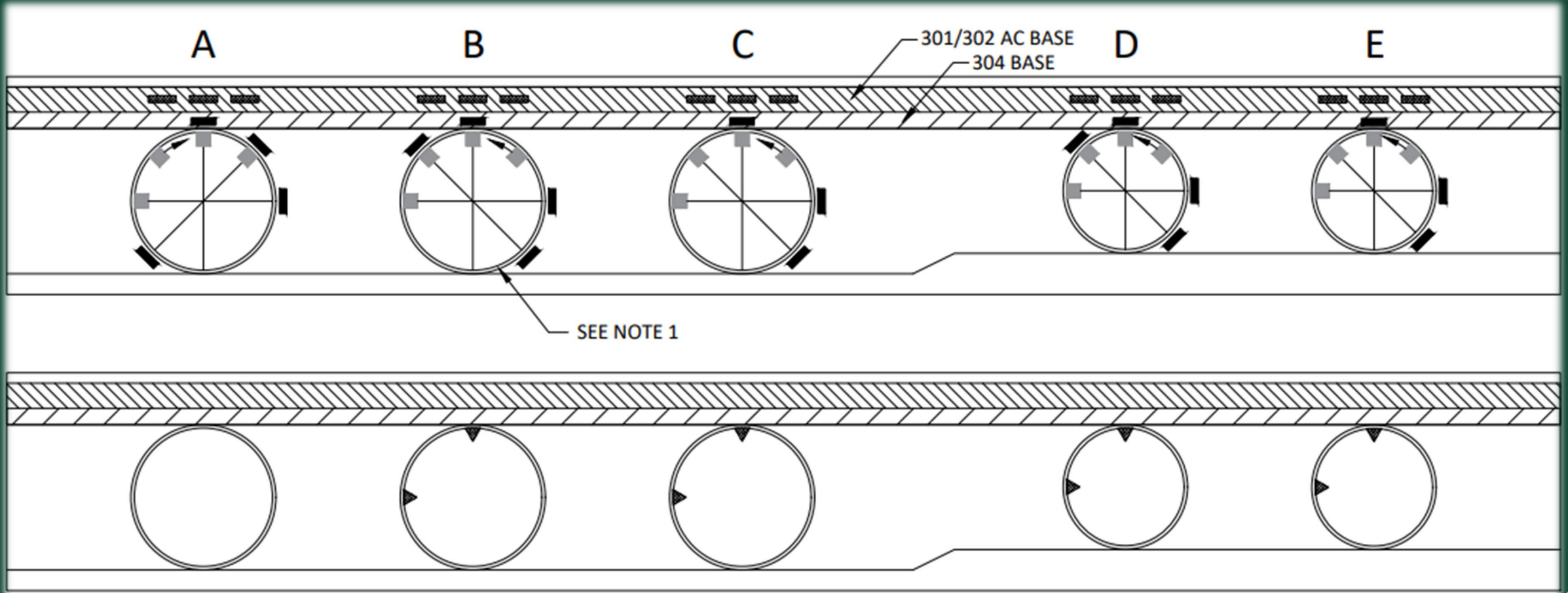


Instrumentation - Typical Layout



Instrumentation Location Plan

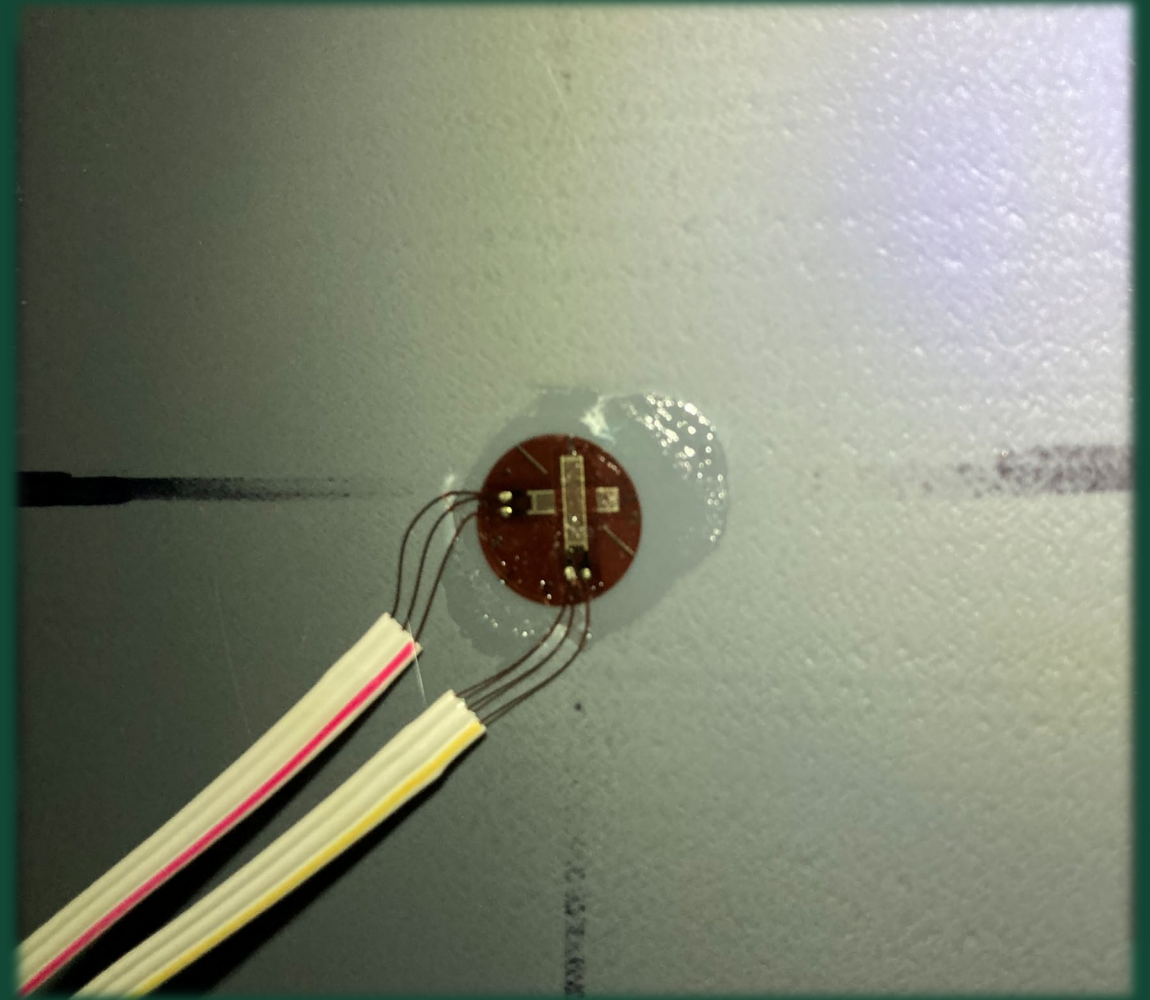
- PRESSURE CELL (18)
- ▨ STRAIN GAGE - ASPHALT (15)
- ▲ STRAIN GAUGE - PIPE (8)
- STRING POTENTIOMETER (24)



Pipe Instrumentation

Strain Gages (SG)

- Biaxial gages placed at crown and springline on interior pipes
- Placed on corrugation valley to prevent liner interference

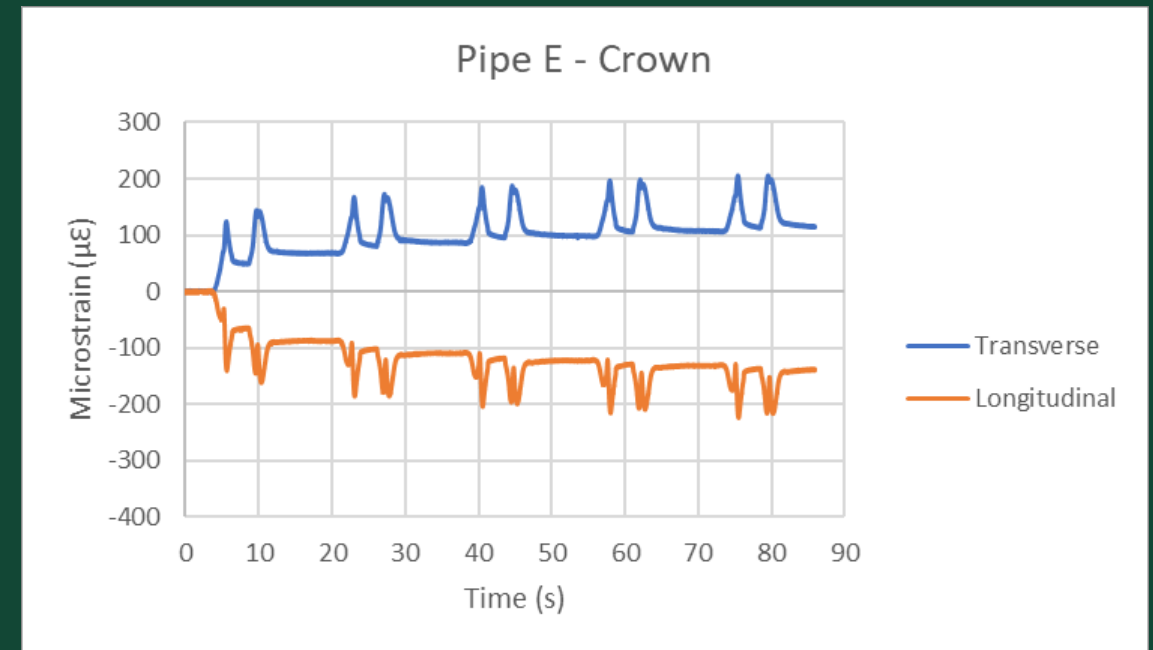
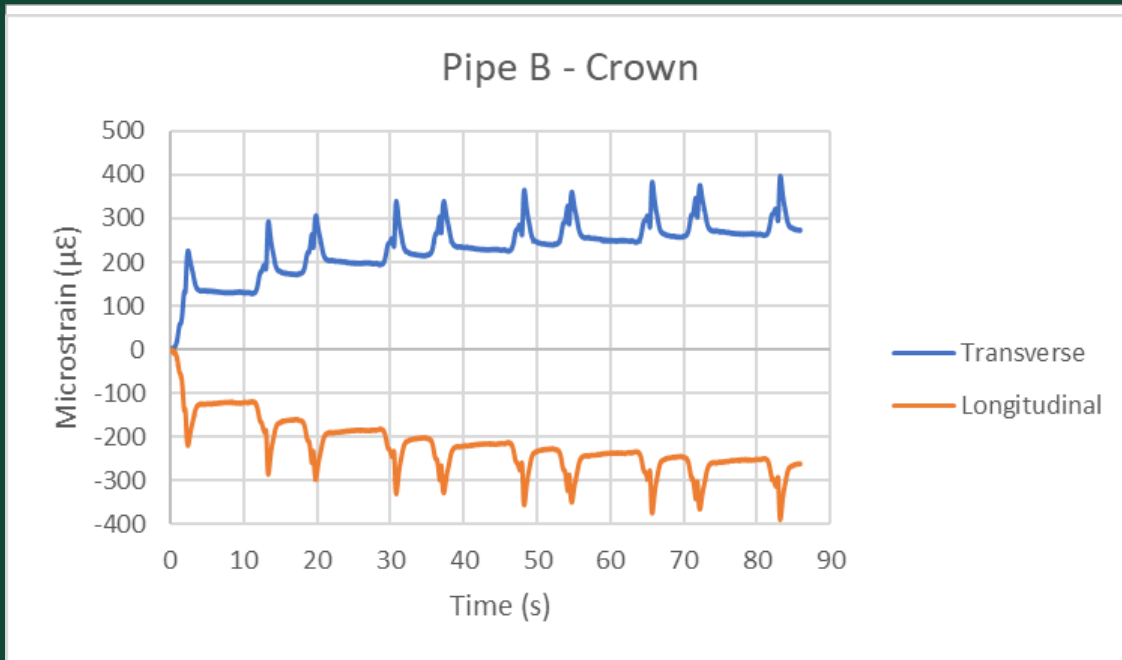


Early Strain Behavior

304

vs.

#57

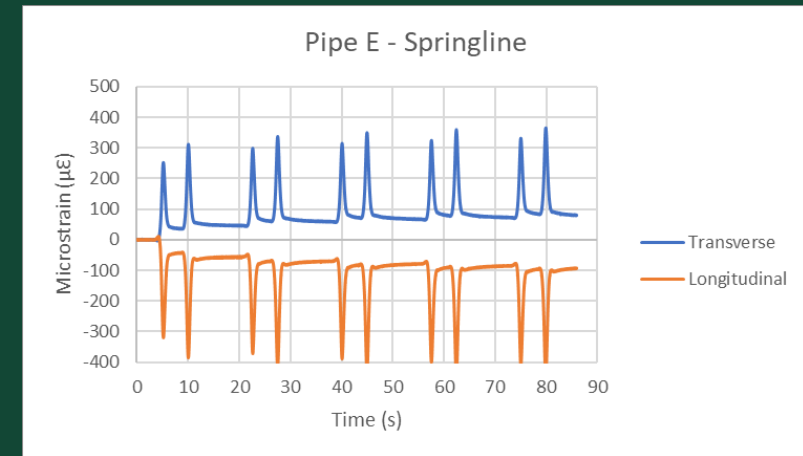
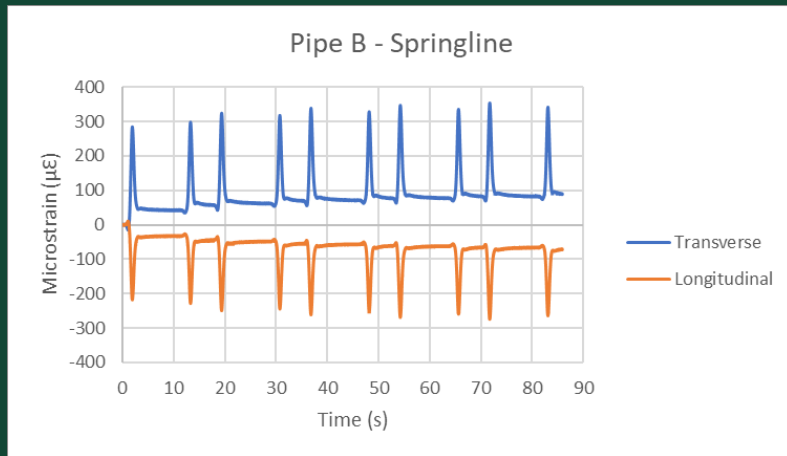
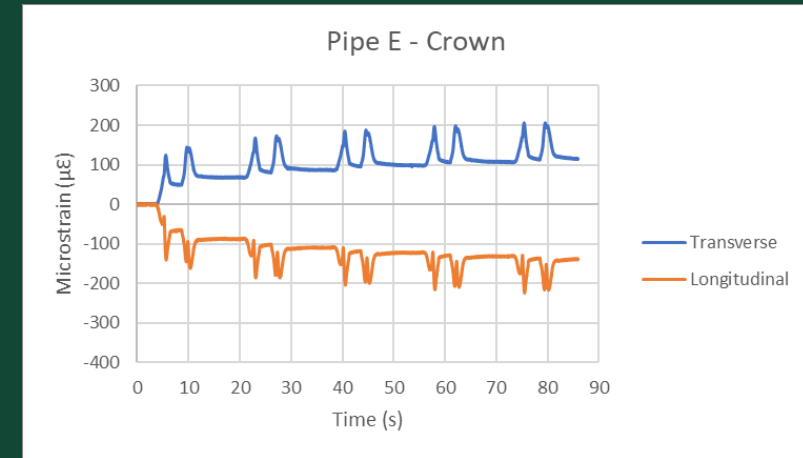
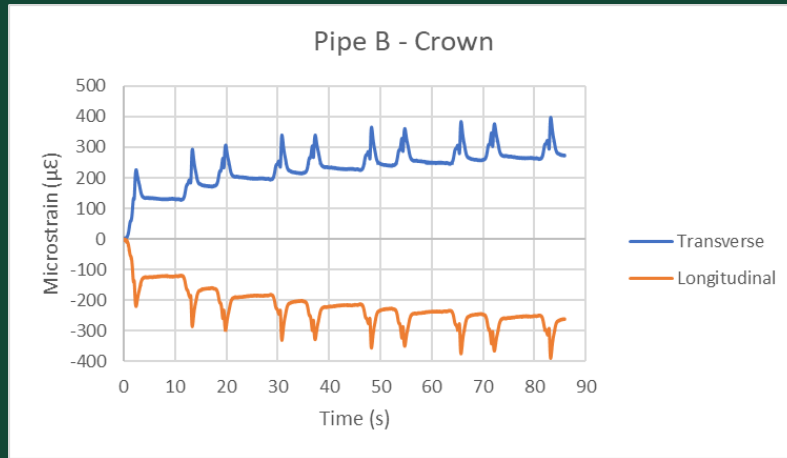


Early Strain Behavior → Equilibrium

304

vs.

#57

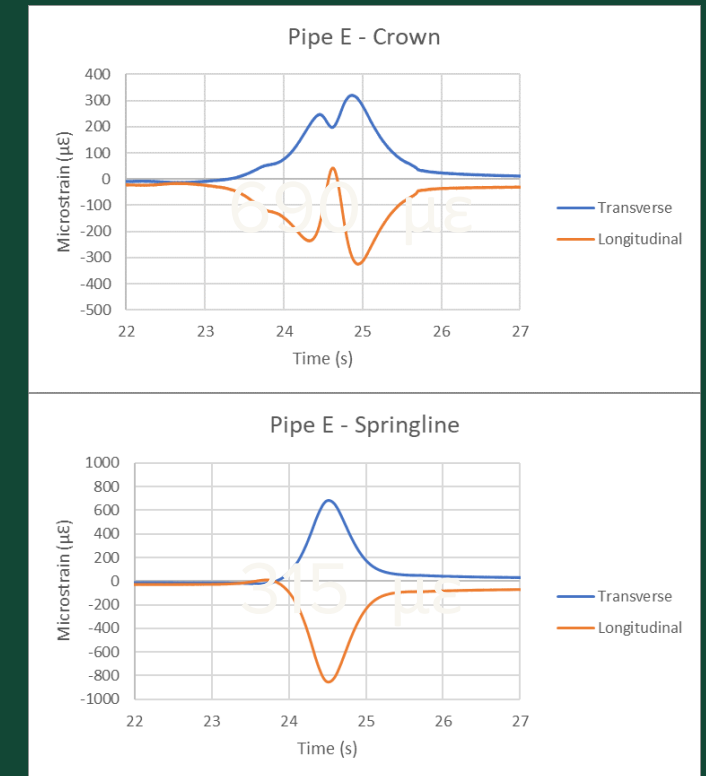
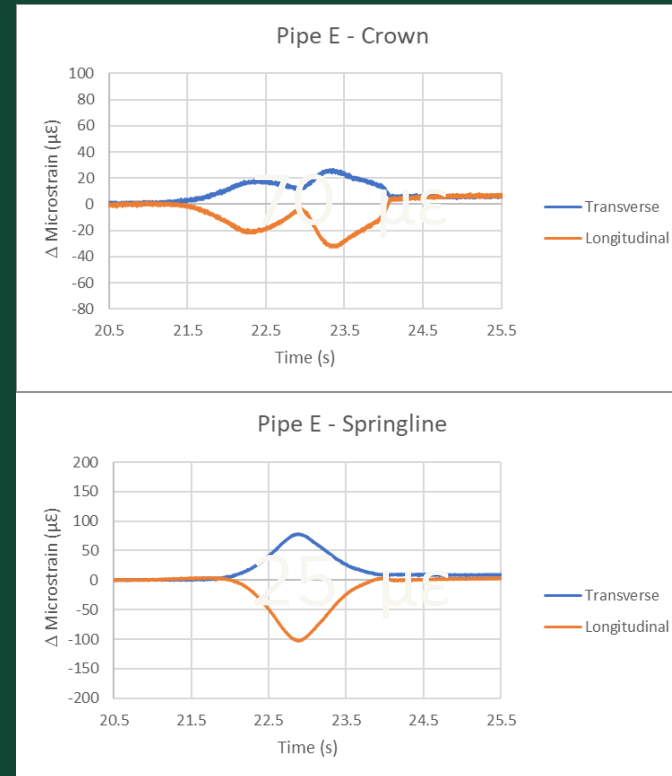
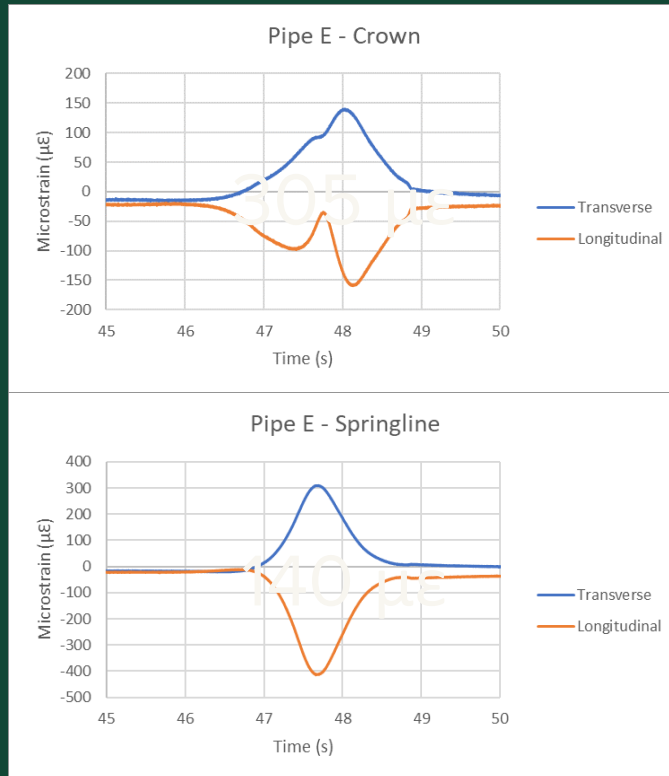


Temperature Response – Pipe Strain

10k - ROOM

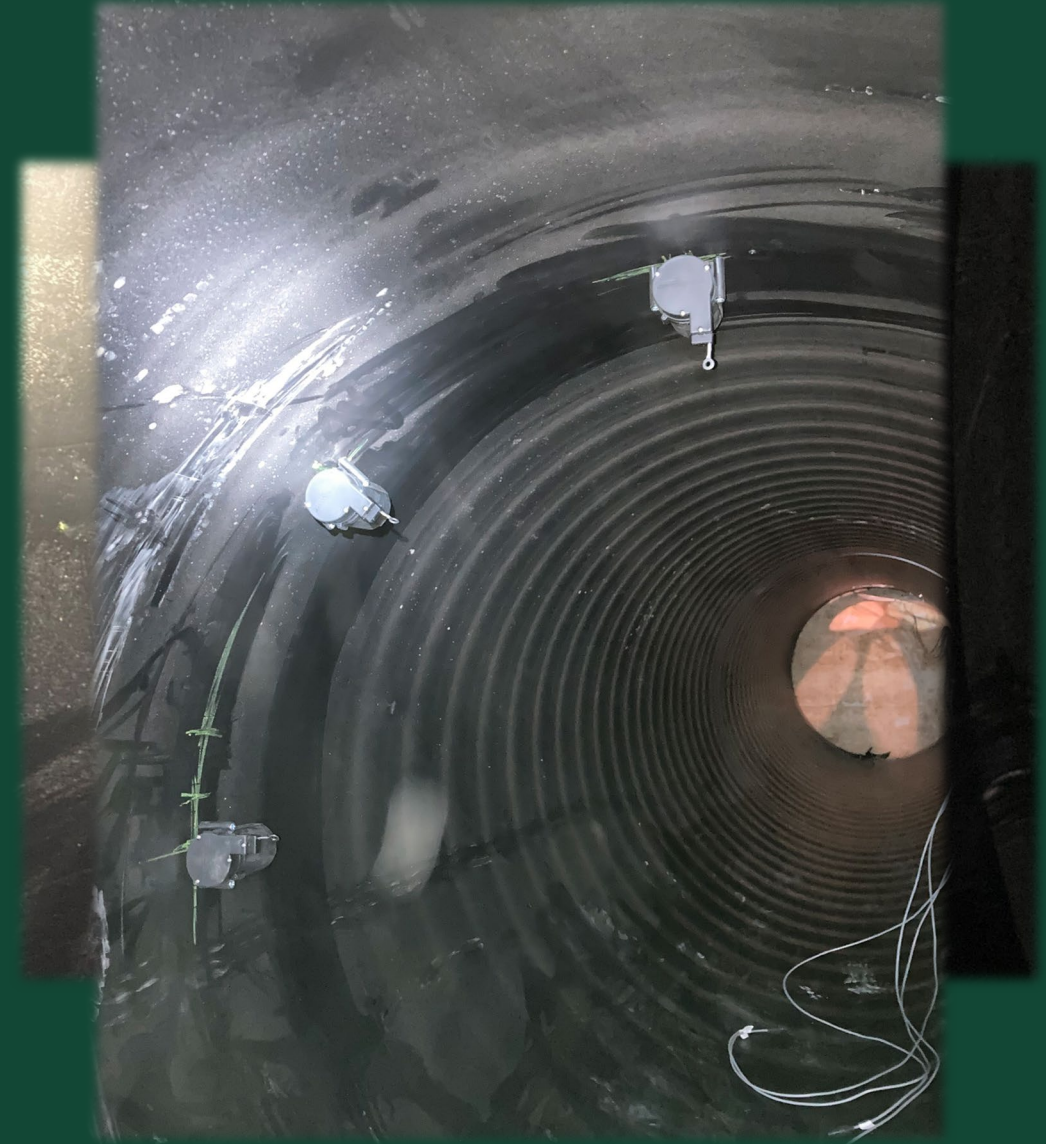
30k - COLD

80k - HOT



Pipe Instrumentation

- String Potentiometers (SP)
 - 4.7 inch range potentiometers placed at crown, arch/haunch, springline and circumferential locations
 - Fixed to corrugation valley
- Thermocouples (TC)
 - Placed at invert and crown locations

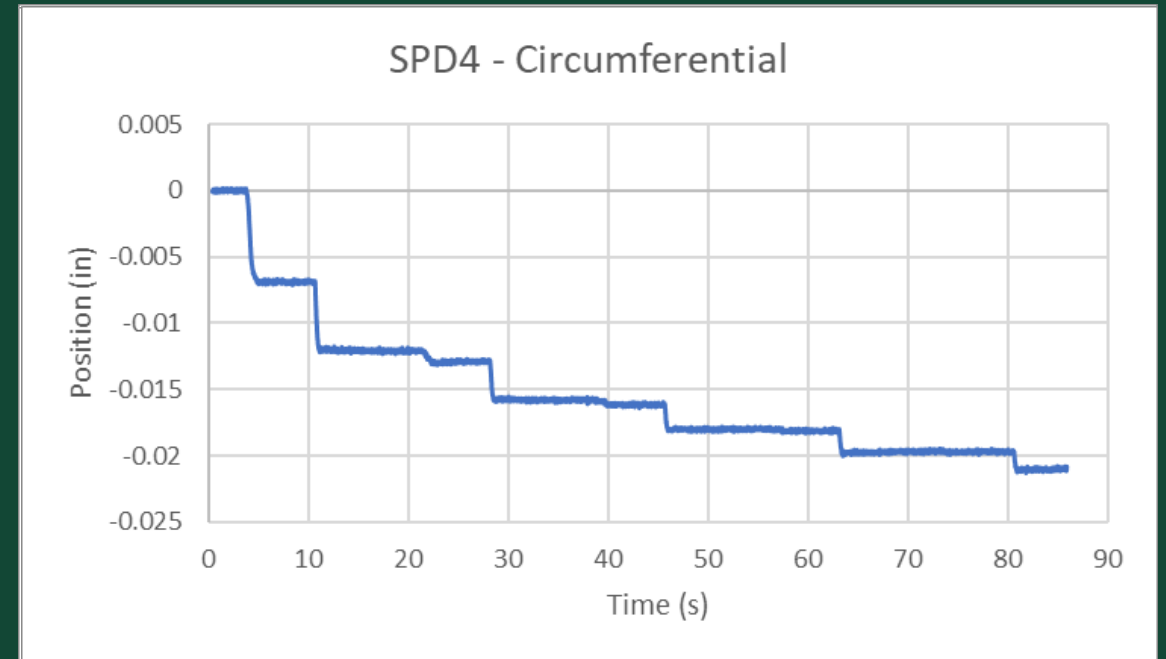
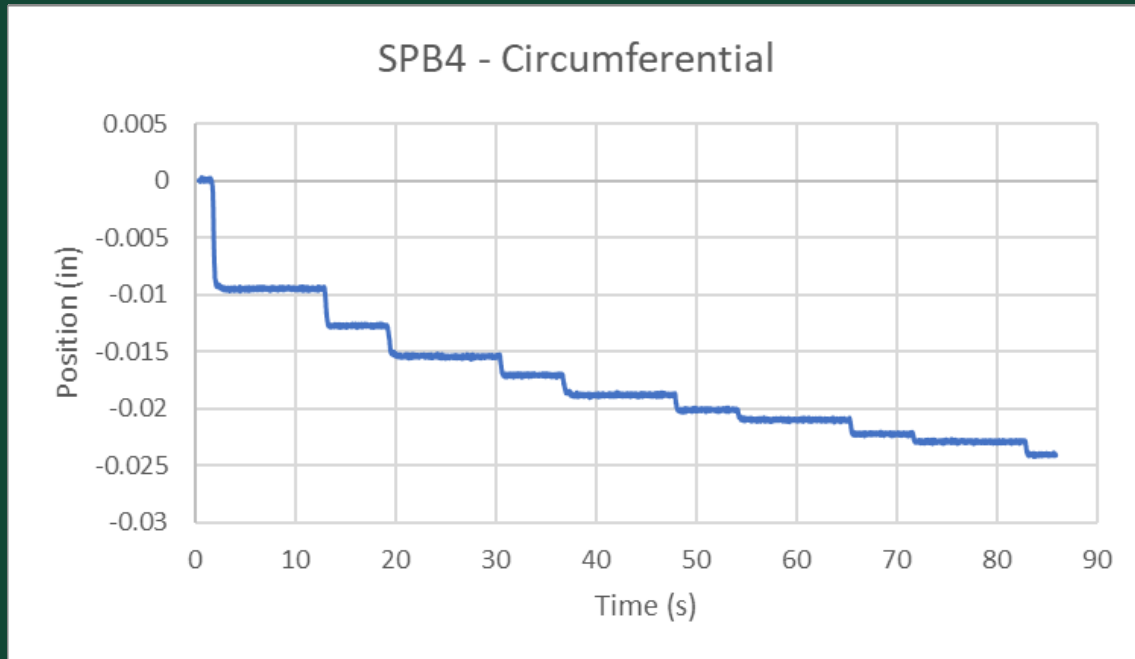


Early Deflection Behavior – 100 Load Cycles

304

vs.

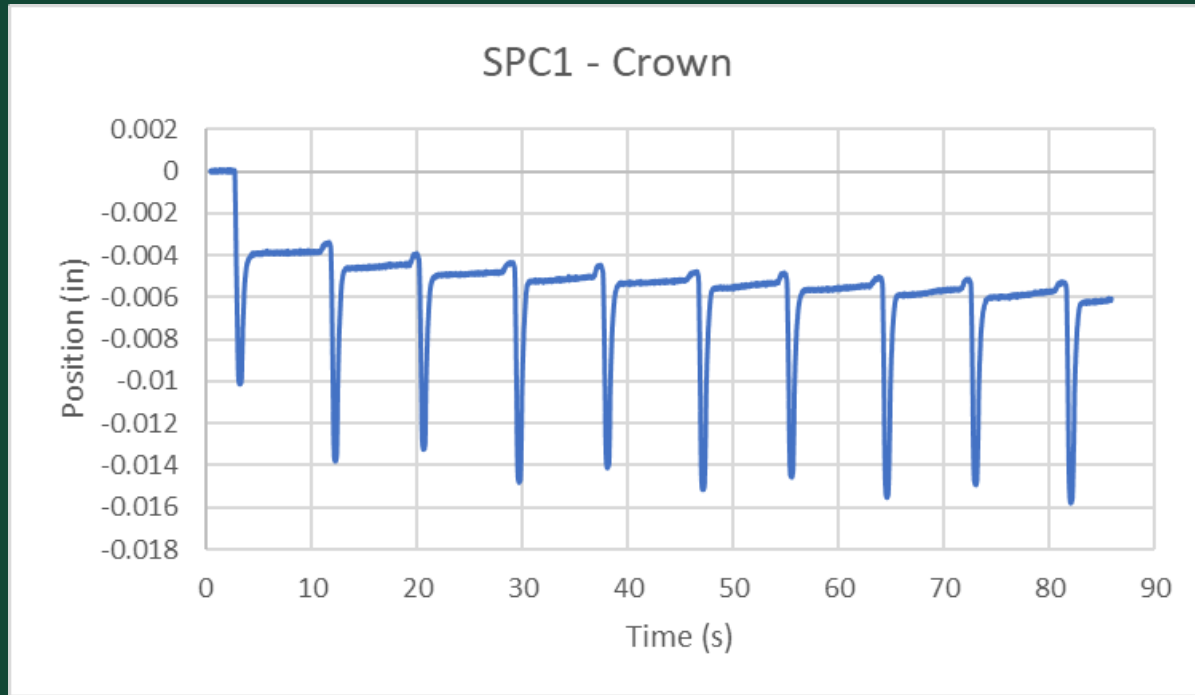
#57



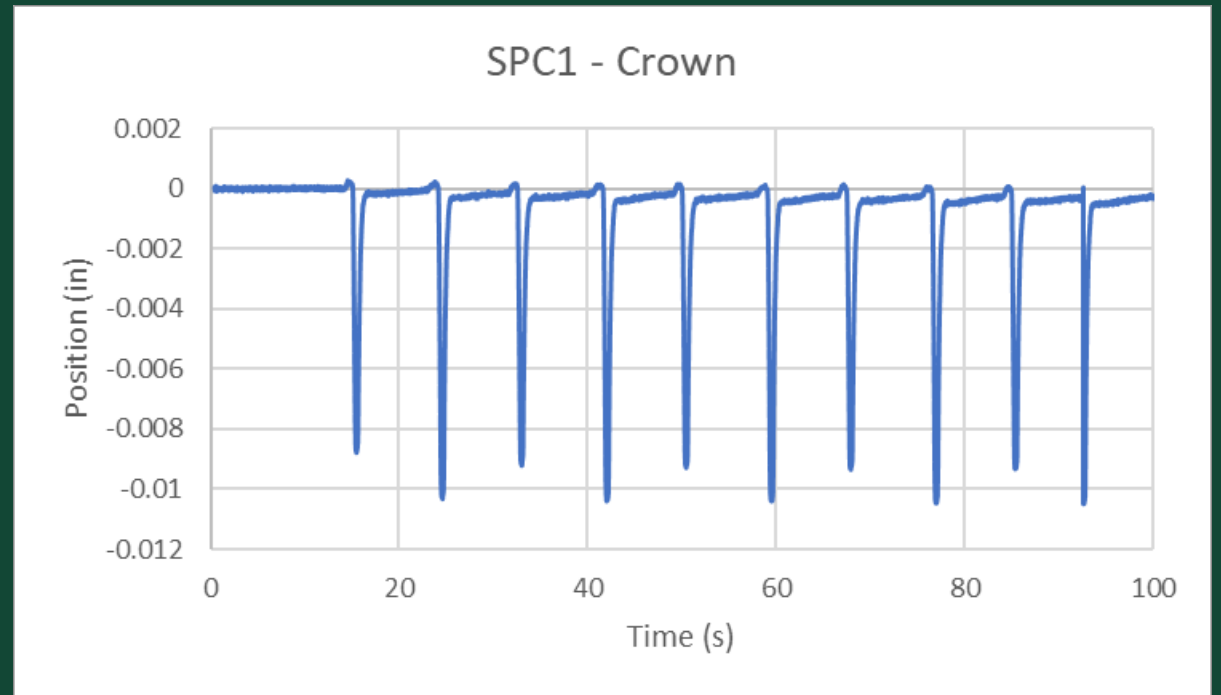
ALL VALUES VERY SMALL

Early Deflection Behavior → Equilibrium

100 load cycles



1,000 load cycles



Temperature Response – Deflection

10k
ROOM

0.0015 in.

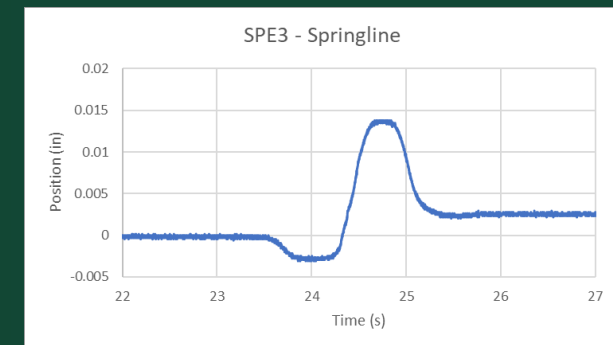
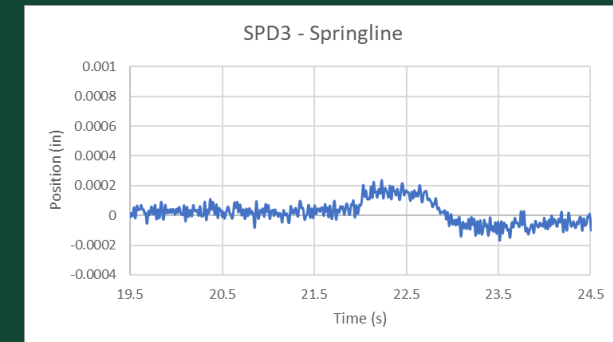
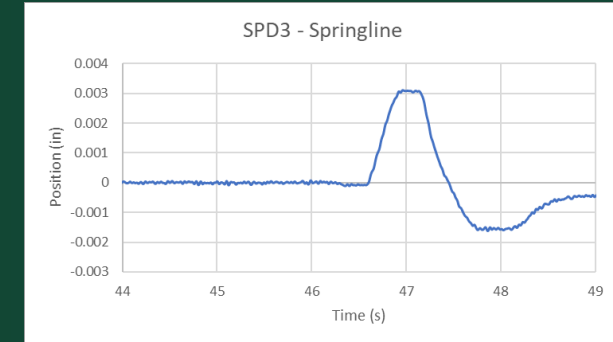
0.011 in. 0.003 in.

30k
COLD

0.015 in.

0.04 in. 0.015 in.

80k
HOT



Embedment & Asphalt Instrumentation

- Pressure Cells (PC)
 - In embedment material at crown, springline, haunch and arch locations
 - Placed in thin layer of sand to avoid potential load concentrations
- Asphalt Strain Gages (SGA)
 - Bottom of asphalt base layer, on top of compacted 304 layer
 - Longitudinal, above pipe crown
- Thermocouples (TC)
 - Bottom and top of asphalt base

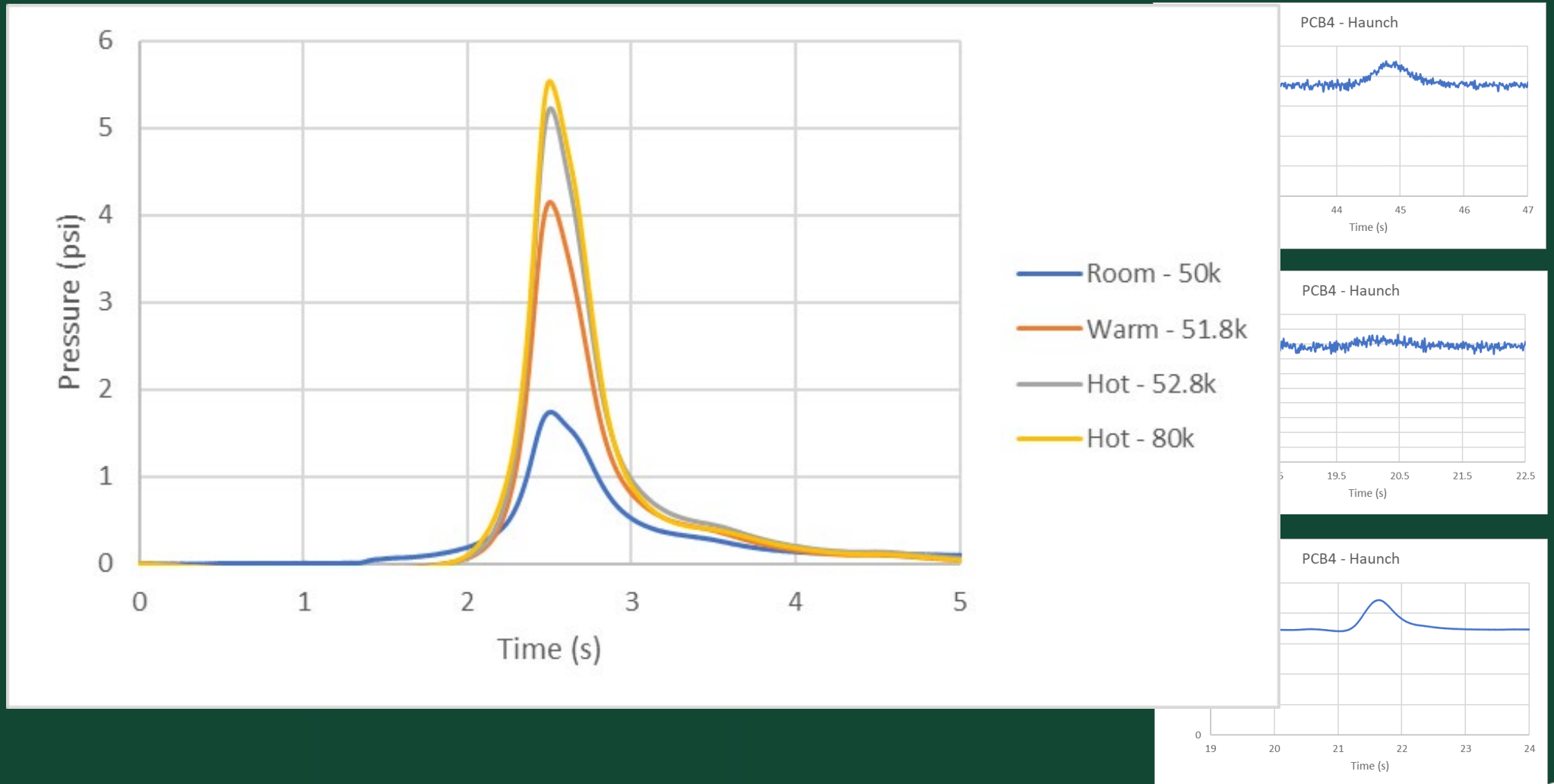


Temperature Response – Pressure

10k
ROOM

30k
COLD

80k
HOT



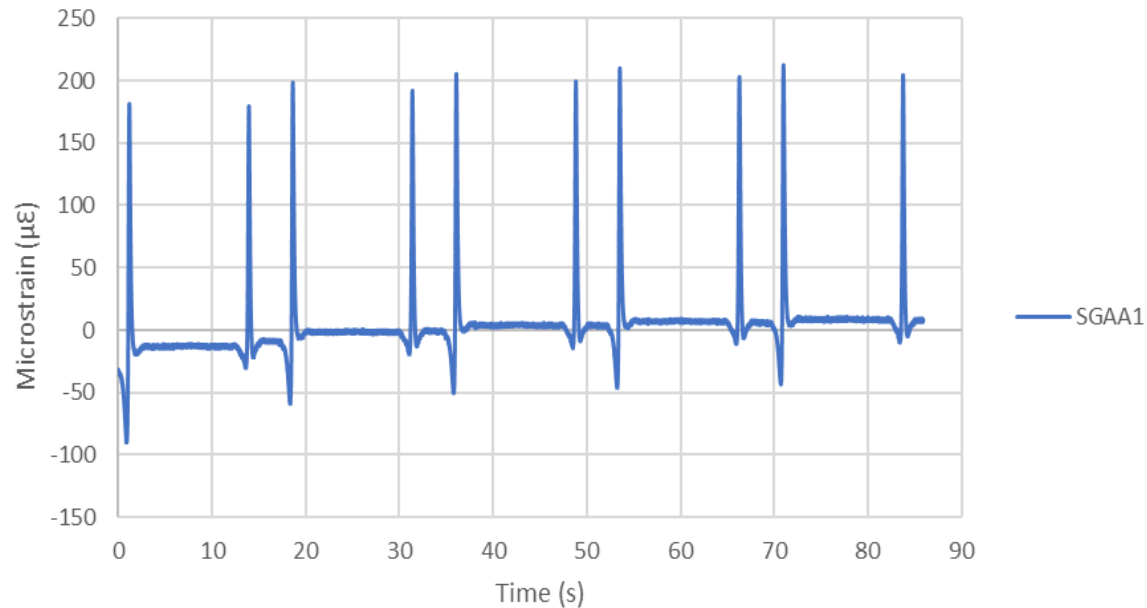
Early Asphalt Strain Behavior

304

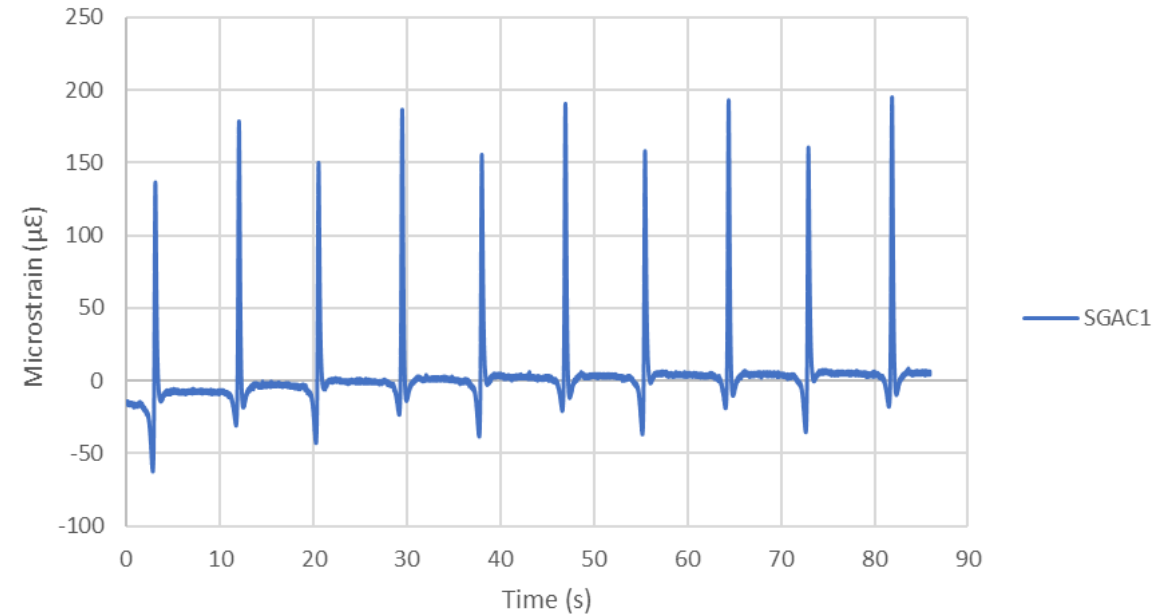
vs.

#57

Asphalt Strain - Pipe A



Asphalt Strain - Pipe C

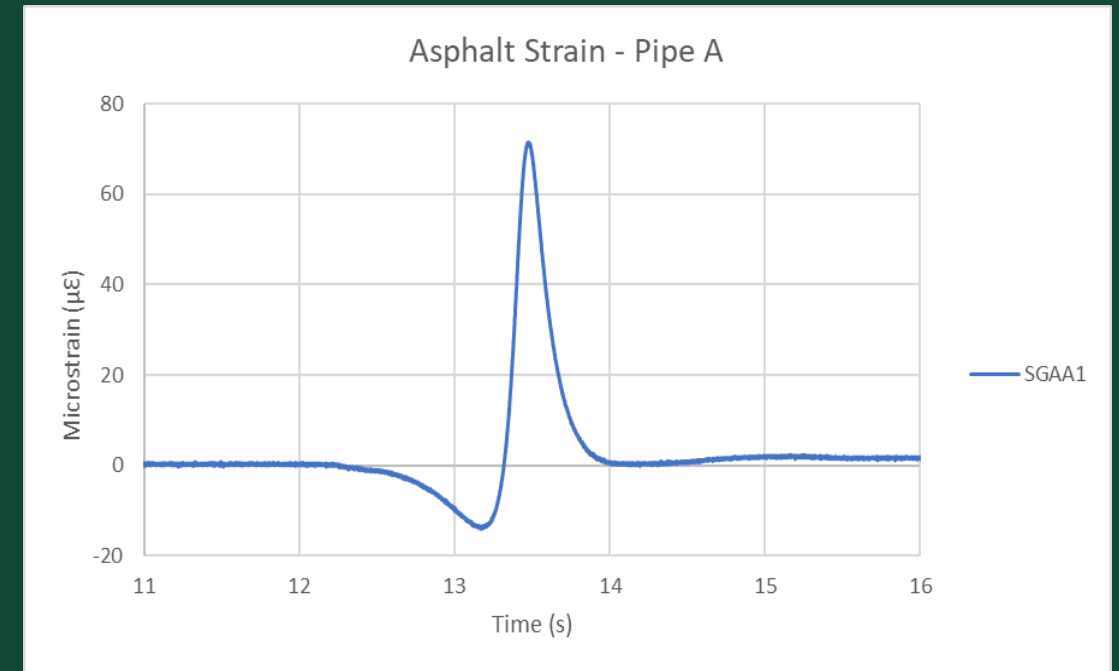
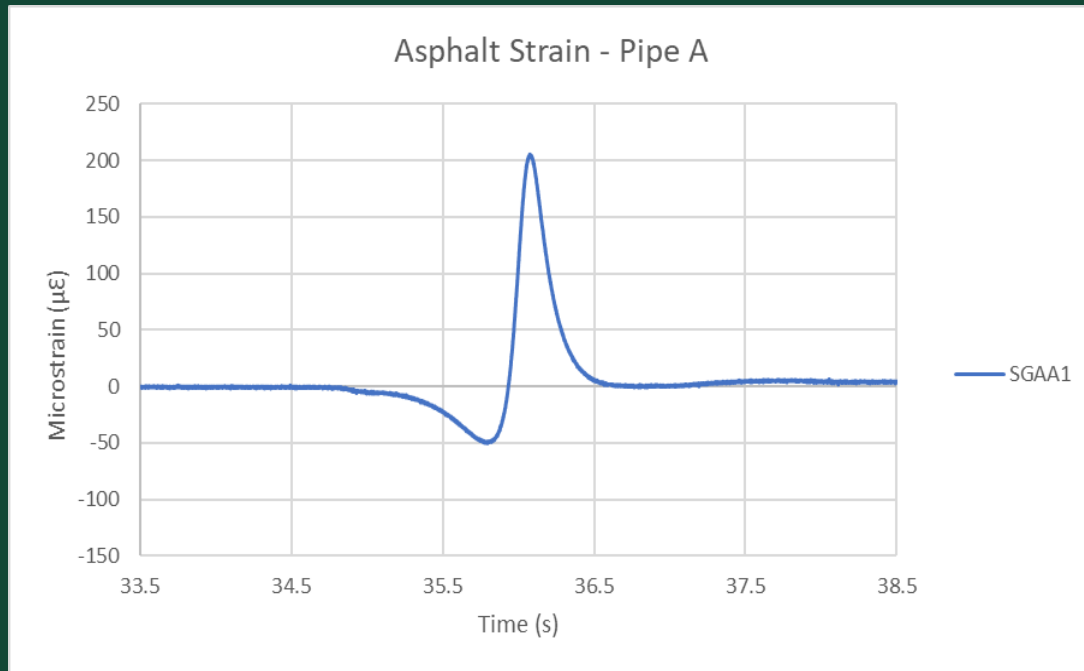


Early Asphalt Strain Behavior → Equilibrium

100 Load Cycles
200 $\mu\epsilon$

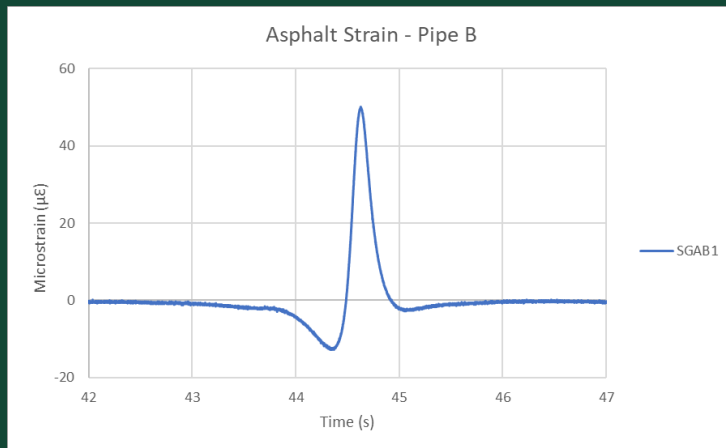
60% + DROP

1000 Load Cycles
75 $\mu\epsilon$



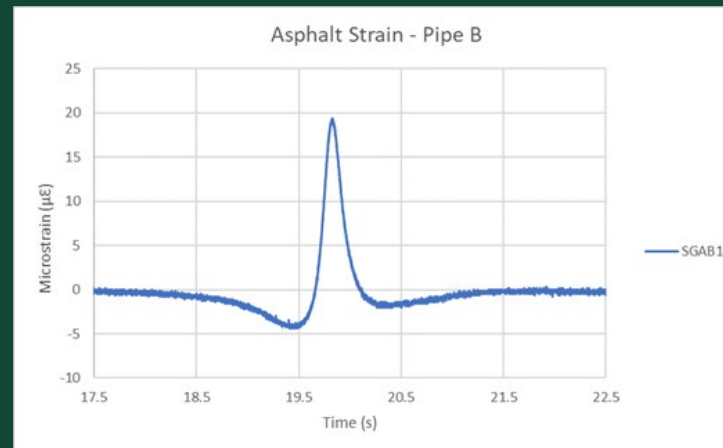
Temperature Response – Asphalt Strain

10k - ROOM



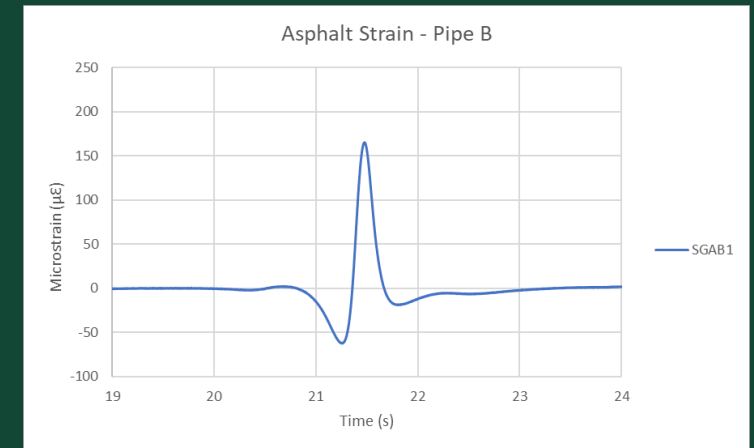
50 $\mu\epsilon$

30k - COLD



20 $\mu\epsilon$

80k - HOT



160 $\mu\epsilon$

Embedment & Asphalt Instrumentation

- Pressure Cells (PC)
 - In embedment material at crown, springline, haunch and arch locations
 - Placed in thin layer of sand to avoid potential load concentrations



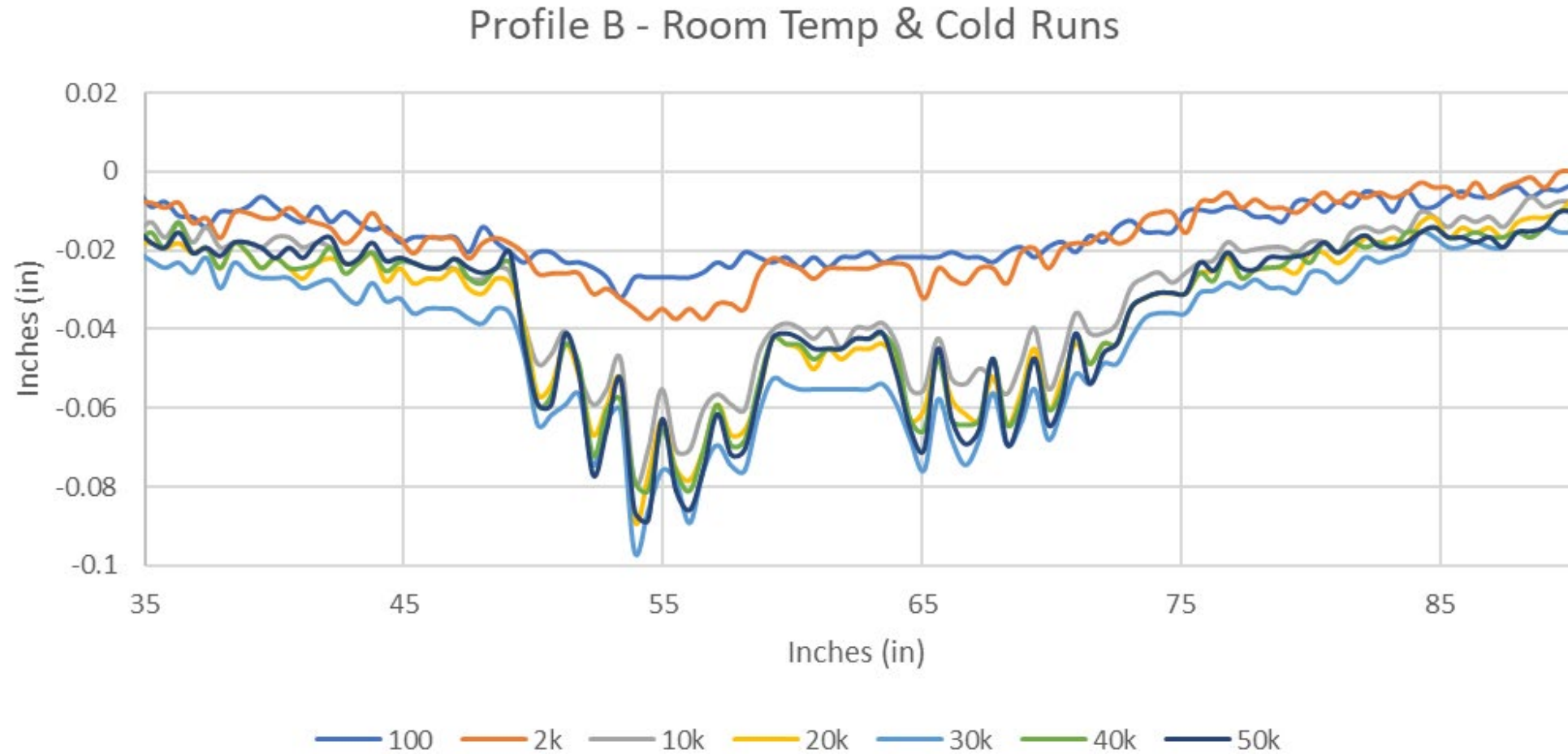
Asph

- Measurement of midpoints
- Carriage travel
- Onboard data collection and elevations
- Elevation measurement



Asphalt Profile Results

- R
- D
- P



Summary

- No major differences between embedment materials
- No large differences in behavior between two pipe materials, across the temperature range tested
- Despite not achieving 2ft of cover requirement (AASHTO LRFD), all pipes performed very well
- Never approached anywhere near 5% deflection limit, even under most extreme temperatures (0.05" MAX)

Thermoplastic Pipes Performance at High and Ambient Temperatures Under Shallow Cover



Presenter:
AbdulBastDahar
PhD Student
Ohio University

Presentation Layout

- Introduction
- Literature Review
- Research Gap
- Objectives
- Materials and Methods
- Results

Background

- **Recent Wildfire Devastation:** California's wildfires have intensified in recent years, causing extensive damage to both forests and developed regions.
- **Historic Scale of Destruction:** Since 2015, the state has witnessed 15 of its 20 most destructive fires, leading to widespread annihilation of homes and buildings across diverse landscapes, from the Sierra Nevada foothills to the Coast Range.
- **Intersection of Human Habitation and Fire Zones:** The fires are increasingly occurring in populated areas— or conversely, more people are settling in fire-prone regions— introducing new risks for health and infrastructure.
- **Post-Fire Hazards:** Among the various dangers that follow in the aftermath of these fires are buried infrastructure, including HDPE (High-Density Polyethylene) and PP (Polypropylene) pipes. These materials are commonly used for utilities and are at risk of damage.
- **Infrastructure at Risk:** These buried pipes face threats from both static and moving loads beneath pavements, which can be compromised or weakened by the intense heat and subsequent environmental changes caused by the fires.



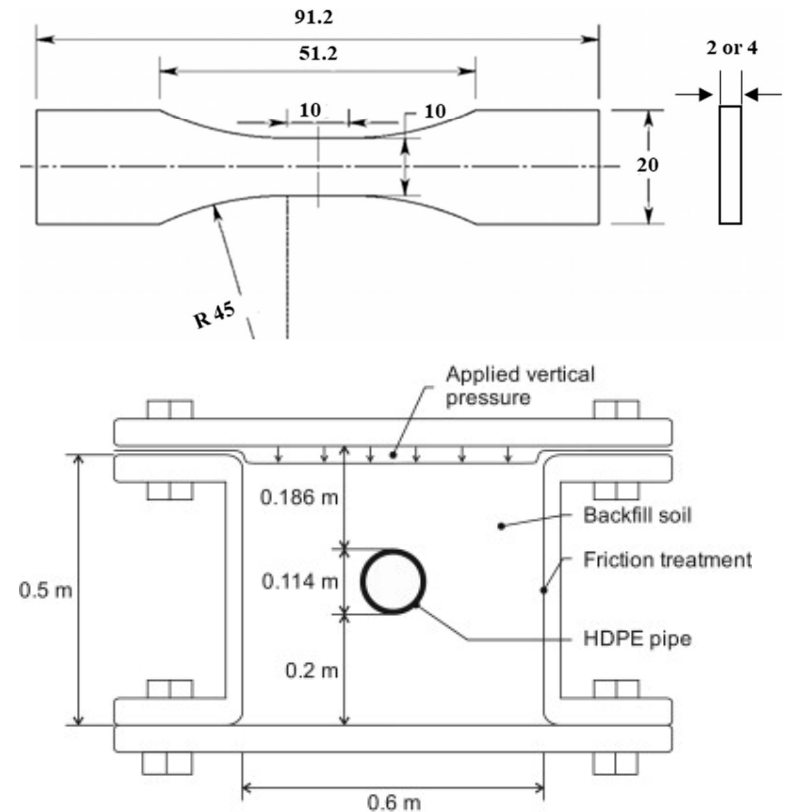
Literature Review

Material Level Testing

- Povob et al, 1996
- R W .Bonds, 2000
- Merah et al, 2003
- Amjadi & Fatemi, 2021

System Level Testing

- Krushchitzky & Brachman, 2013
- Zhang et al, 2022



Literature Review

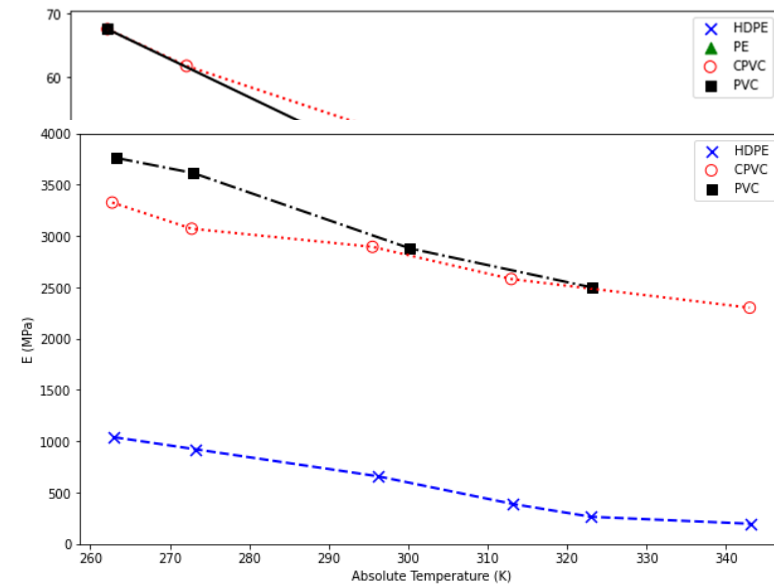
- Yield stress decreases linearly with temperature increase from 32 MPa at -10 °C (263.15 K) to 9 MPa at 70 °C (343.15 K).
- Ductile fracture is the predominant failure mechanism, with notable necking at room and high temperatures.
- Linear correlation between yield strength of HDPE and temperature:
- Formula:

$$\sigma_y = 112.85 - 0.305T \quad 263\text{K} \leq T \leq 343\text{K}$$

- Theoretical support from Eyring's theory of viscosity.

$$\sigma_y = \frac{H}{V} + \frac{R}{V} \ln\left(\frac{\dot{\epsilon}}{A}\right) T$$

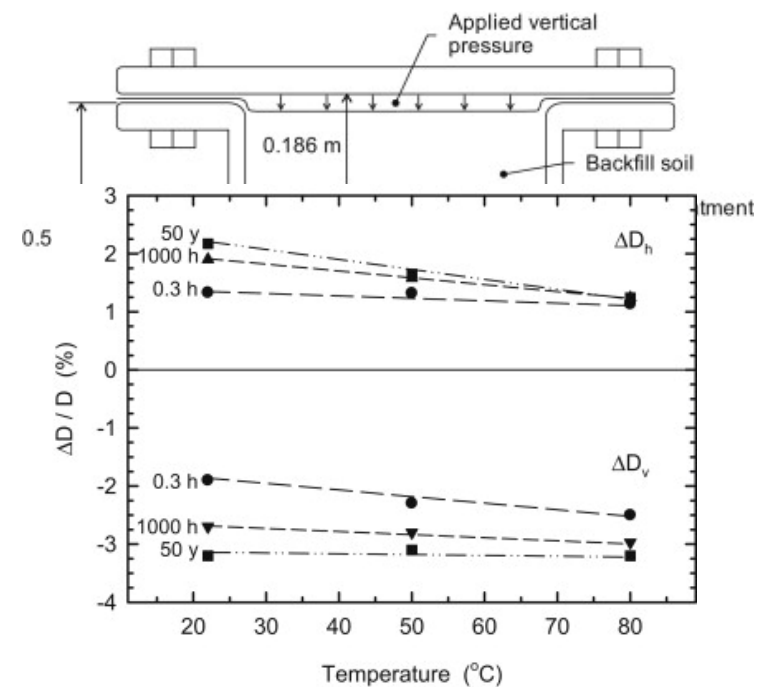
* Reference: Adapted from Merah et al., 2003; R.W. Bonds, 2000; Povob et al., 1996.



Literature Review

Krushehitzky & Brachman, 2013

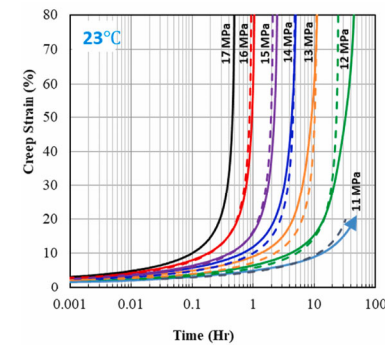
- Significant findings indicate HDPE pipe deflections increase with temperature.
- The study observed vertical deflections in sand-backfilled HDPE pipes increase by 13 times when temperature rose from 22 °C to 80 °C.
- This increase in deflection is attributed to greater circumferential compression at higher temperatures.
- Even under prolonged pressure for 1000 hours, vertical pipe deflections continued to rise, although at a slower rate.



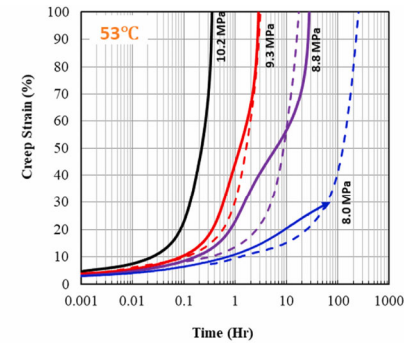
Literature Review

Amjadi & Fatemi, 2021

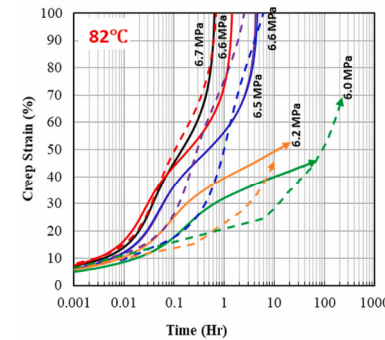
- The study revealed a decrease in creep strength and an increase in both creep strain and strain rate with rising temperature.
- The Larson-Miller parameter, typically used for metals, effectively correlated the time to rupture, stress, and temperature data for HDPE.
- The Monkman-Grant relation and the Findley power law were successfully applied to model the HDPE's creep behavior.
- The research also included long-term creep tests at room temperature, validating the extrapolation accuracy of short-term creep results to predict long-term creep life.



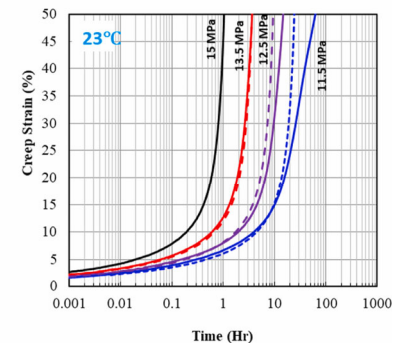
(a)



(b)



(c)

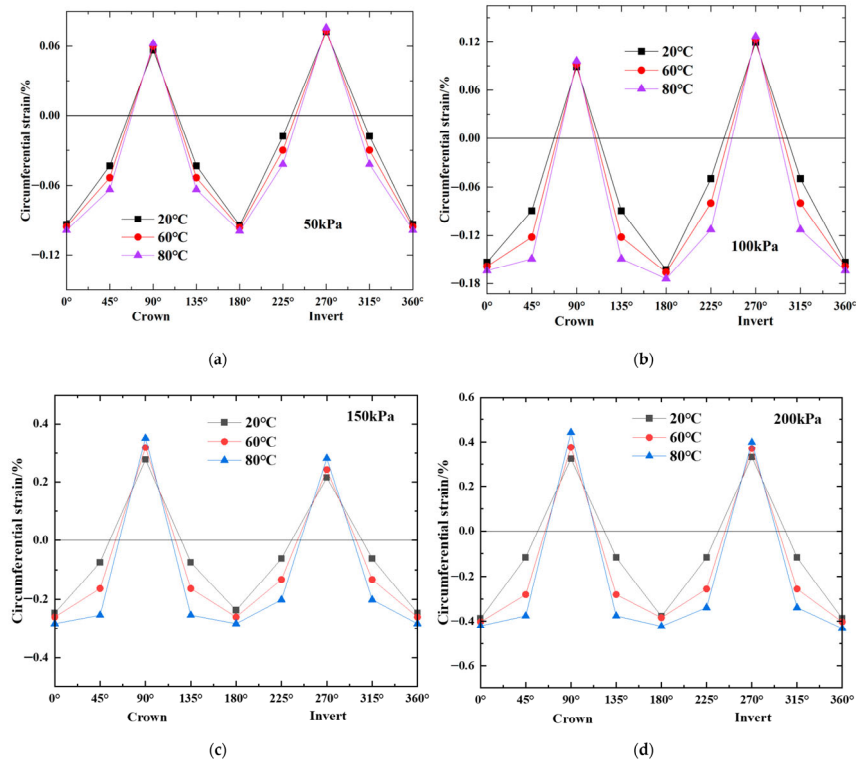


(d)

Literature Review

Zhang et al, 2022

- Sequential pressure and temperature variations impact deformation and strain in HDPE pipes.
- Findings suggest alterations in pipe deformation and strain distribution under environmental stress.
- Emphasizes the importance of understanding soil stiffness around HDPE pipes.

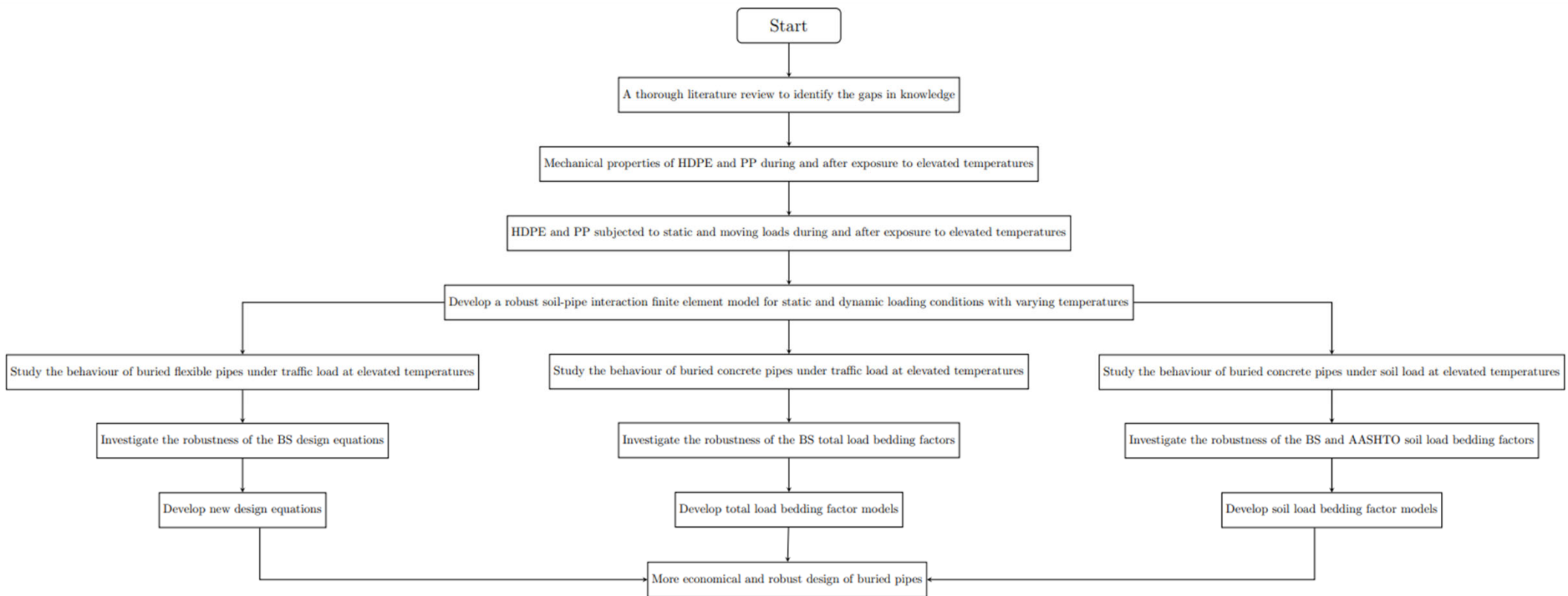


Research Gap

- Limited data of HDPE and PP pipes' mechanical properties during and after exposure to elevated temperatures.
- Limited research on HDPE and PP pipes under shallow covers with static and dynamic loads after exposure to high temperatures.
- Lack of design equations for HDPE and PP pipes considering elevated temperatures.
- Need for guidelines on the thermal interaction between adjacent HDPE and PP pipes.

Objectives

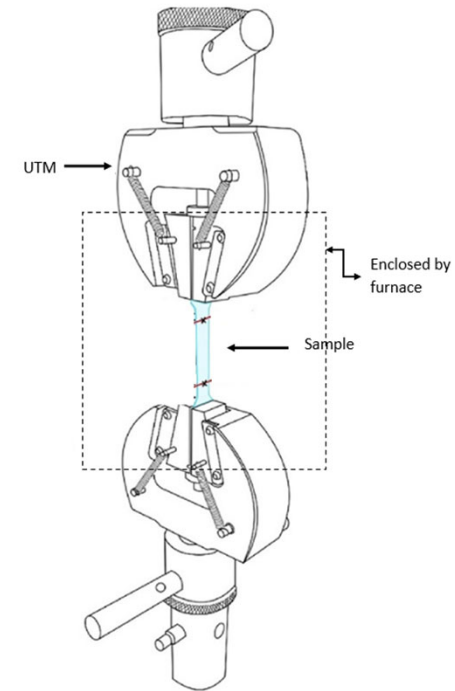
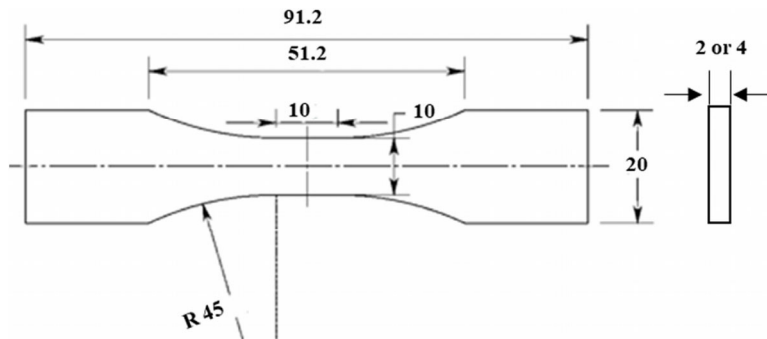
- To investigate the mechanical properties of HDPE and PP pipes across a range of temperatures.
- To examine the response of HDPE and PP pipes under varying loads when installed under shallow cover.
- To develop and validate a full-scale finite element model for HDPE and PP pipes under shallow cover, incorporating a parametric study with traffic load simulations.
- To develop design equations for HDPE and PP pipes that factor in elevated temperatures.
- To establish guidelines for the thermal interaction between closely installed HDPE and PP pipes.



Materials and Methods

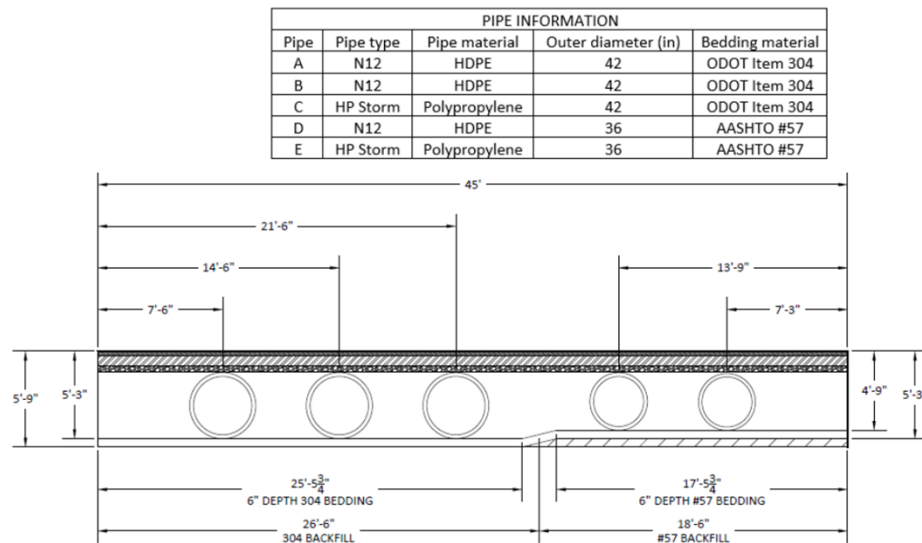
1-Material Level Testing

- Coupons will be subjected to tensile testing while being heated.
- Coupons will undergo tensile testing after cooling from elevated temperatures.

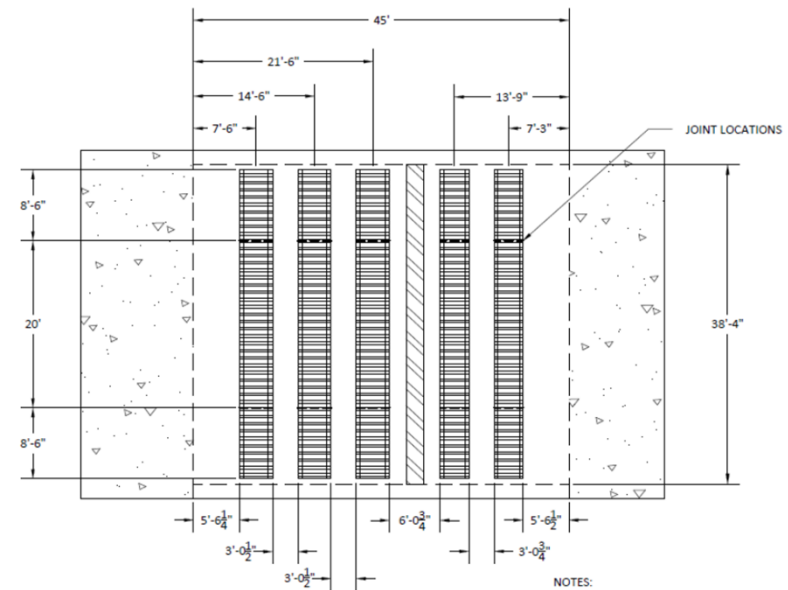


Materials and Methods

2-System Level Testing at the APLF Facility of Ohio University, Lancaster Campus

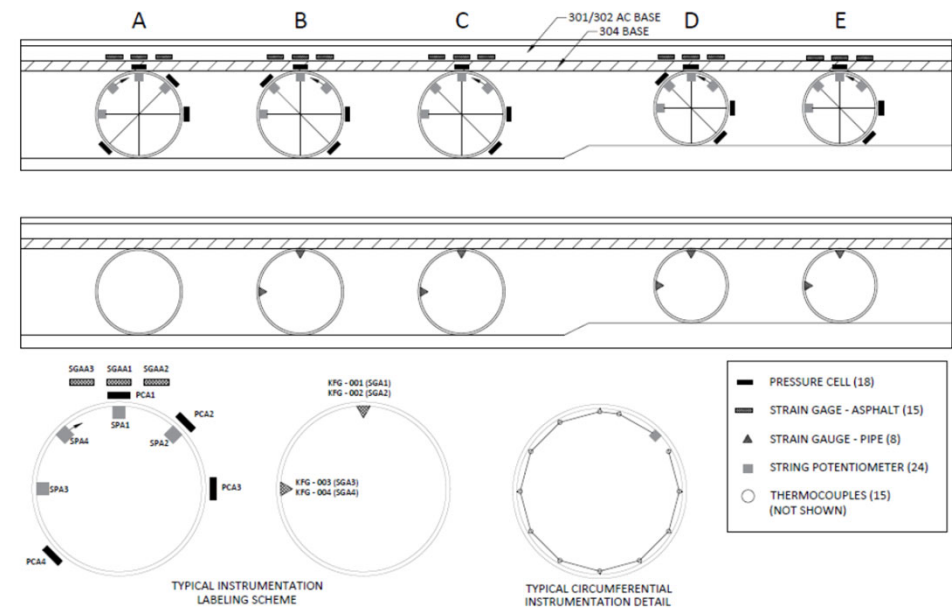
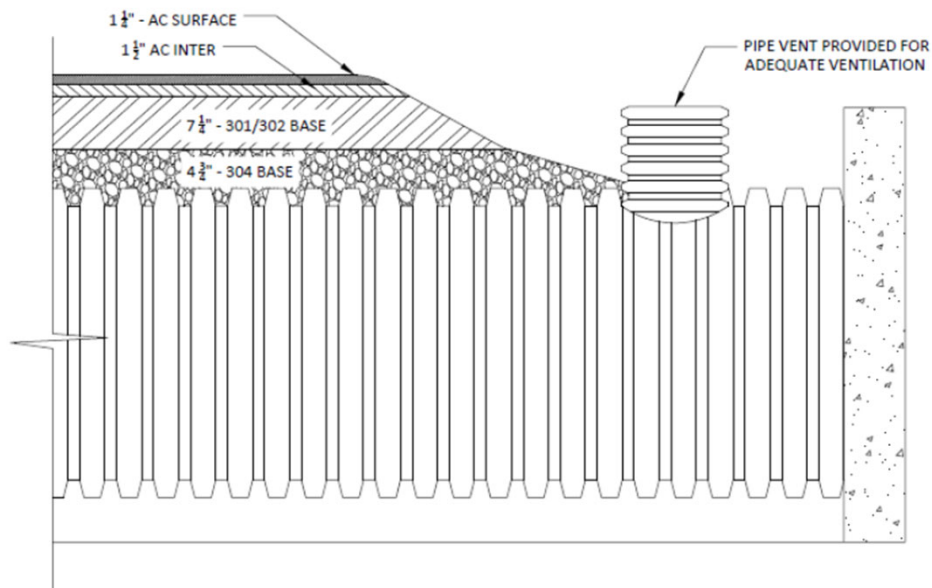


- NOTES:
- 1:4 BEDDING SLOPE PICTURED, BASED ON 2'-0" CLEAR DISTANCE TO PIPES.
 - TOP OF ASPHALT TO BE LEVEL WITH CONCRETE SLAB ON PIT WALLS.
 - HATCHED BEDDING REPRESENTS EXISTING SOIL.

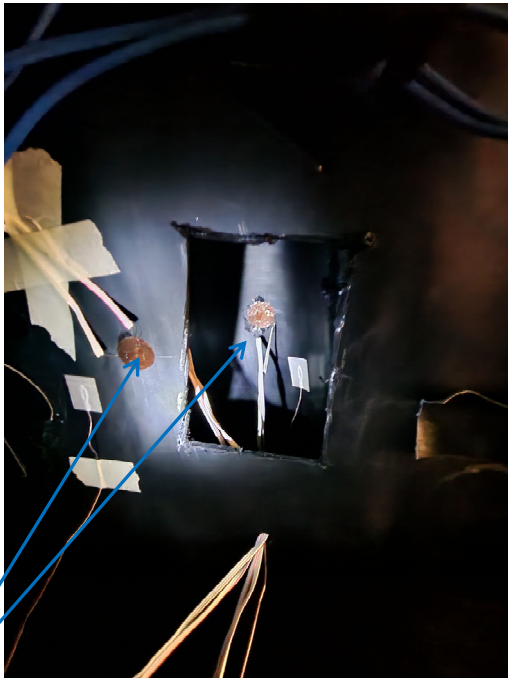


- NOTES:
- BOTTOM DIMENSIONS ARE CLEAR DISTANCE B/W EXTERIOR CORRUGATION WALL.
 - PIPE LENGTH DIMENSIONS BASED ON 8" CLEAR DISTANCE TO WALL. ADJUST AS NEEDED.
 - SLOPED AREA INDICATED BY CROSS-HATCH.

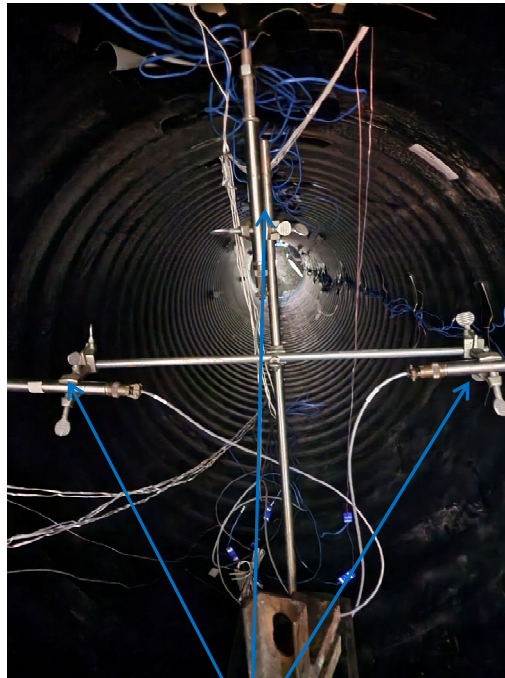
Materials and Methods



Instrumentation Setup



Strain Gauges



LVDTs



Propane Gas Heater

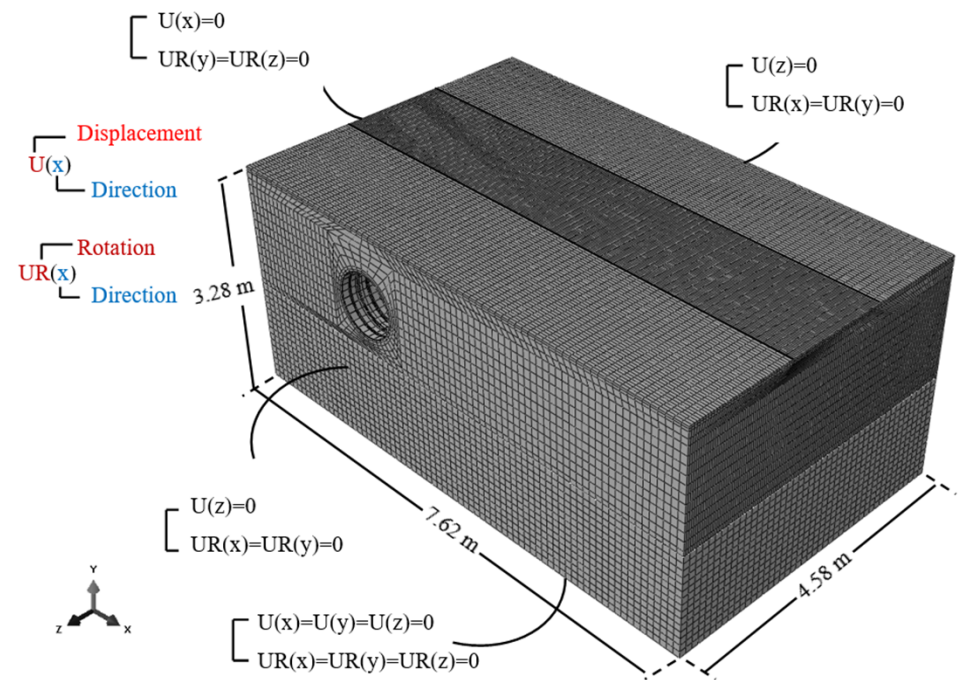
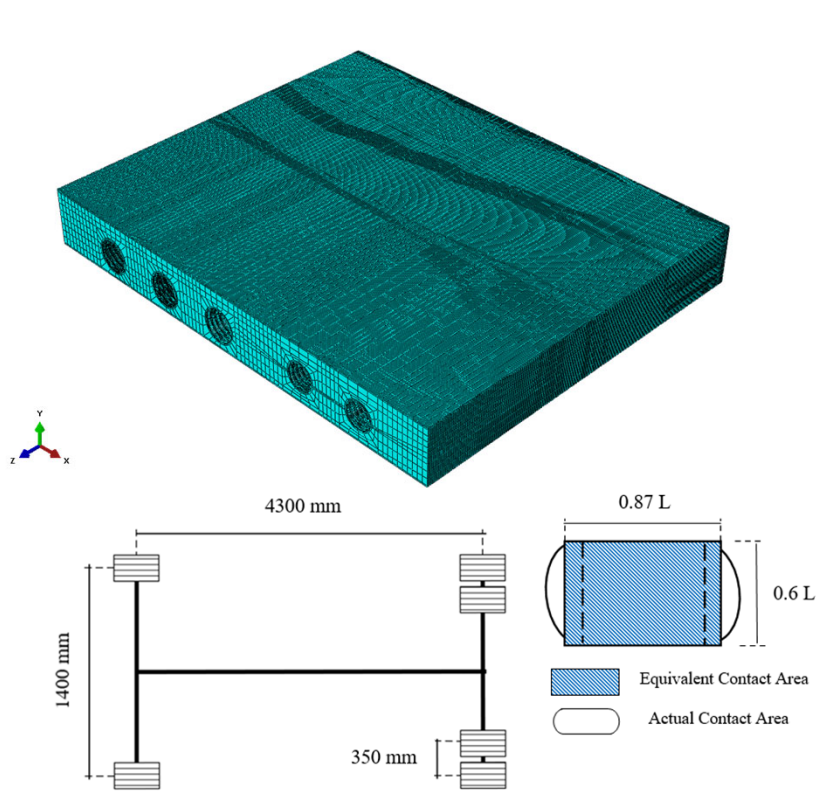
Materials and Methods

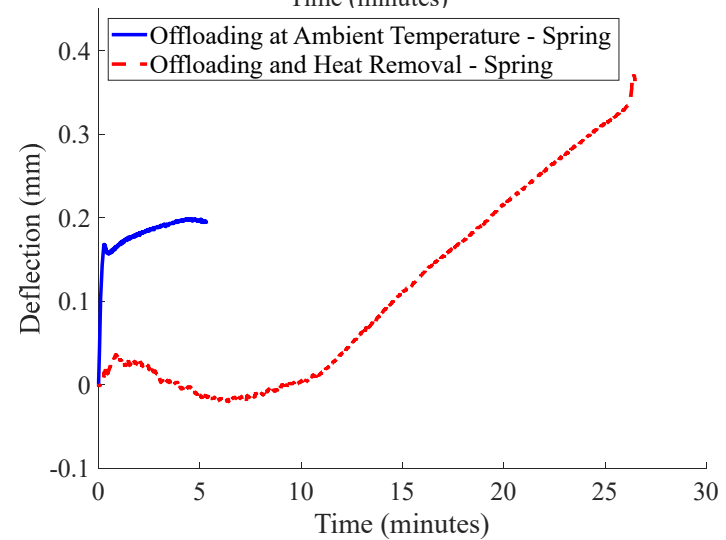
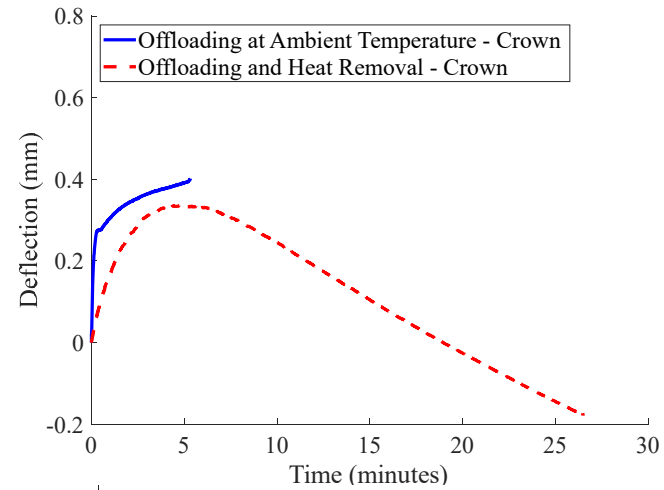
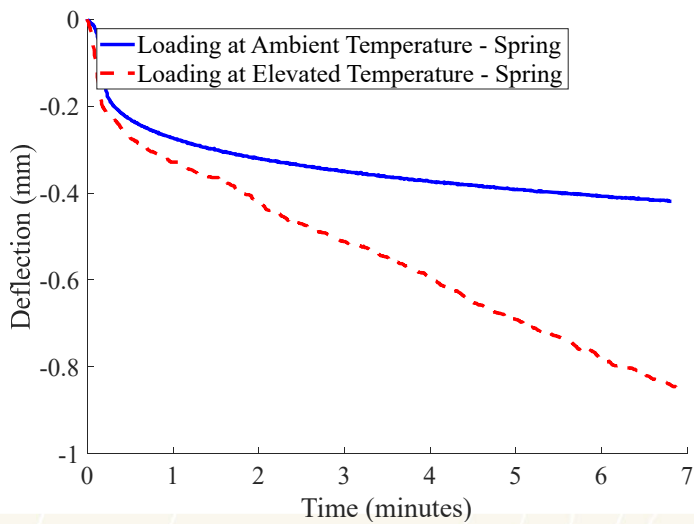
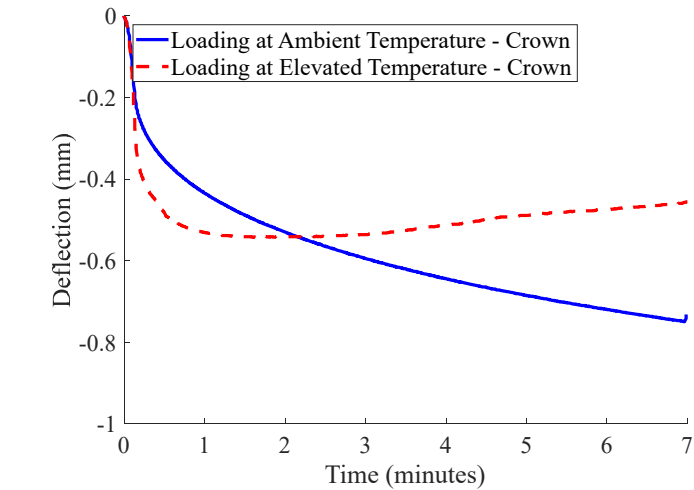
No.	Layer	Material	Unit weight (kg/m ³)	Elastic modulus (MPa)	Poisson's ratio	Parameter	
						Damping ratio, $\zeta = 0.05$	
						α	β
1	AC surface course	SMA13	2,400	Viscoelastic ^a	0.30	0.342	0.0069
2	AC middle course	SBS-AC20	2,400	Viscoelastic ^a	0.30	0.342	0.0069
3	AC bottom course	HE-AC5	2,300	Viscoelastic ^a	0.35	0.342	0.0069
4	Semi-rigid base	CTB	2,100	14,000	0.25	0.342	0.0069
5	Semi-rigid subbase	CTB	2,100	14,000	0.25	0.342	0.0069
6	Cement-stabilized subgrade	CS	1,900	6,000	0.35	0.342	0.0069
7	Natural soil	In-place soil	1,800	120	0.40	0.342	0.0069

Inputs parameters for defining viscoelastic behavior of each asphaltic layer at 10°C						
No.	SMA13		SBS-AC20		HE-AC5	
	τ_i	G_i	τ_i	G_i	τ_i	G_i
	Prony-Dirichlet series for generalized Maxwell model					
1	1.947×10^{-5}	1.593×10^{-1}	1.003×10^{-4}	1.519×10^{-1}	2.921×10^{-5}	2.019×10^{-1}
2	7.661×10^{-4}	1.681×10^{-1}	3.440×10^{-3}	1.326×10^{-1}	9.227×10^{-4}	2.113×10^{-1}
3	3.968×10^{-2}	2.089×10^{-1}	1.651×10^{-1}	1.884×10^{-1}	3.689×10^{-2}	1.775×10^{-1}
4	1.004×10^0	2.022×10^{-1}	2.472×10^0	2.603×10^{-1}	1.248×10^0	1.797×10^{-1}
5	2.267×10^1	1.776×10^{-1}	4.009×10^1	1.789×10^{-1}	6.323×10^1	1.499×10^{-1}
6	1.279×10^3	8.091×10^{-2}	2.857×10^3	6.813×10^{-2}	3.399×10^3	7.165×10^{-2}
Elastic modulus for viscoelasticity (moduli time scale is instantaneous)						
	10,000		15,000		6,500	
Williams-Landel-Ferry equation constants						
C1	23.74		27.061		25.44	
C2	177.80		222.90		198.80	
RMSE	0.11		0.02		0.07	

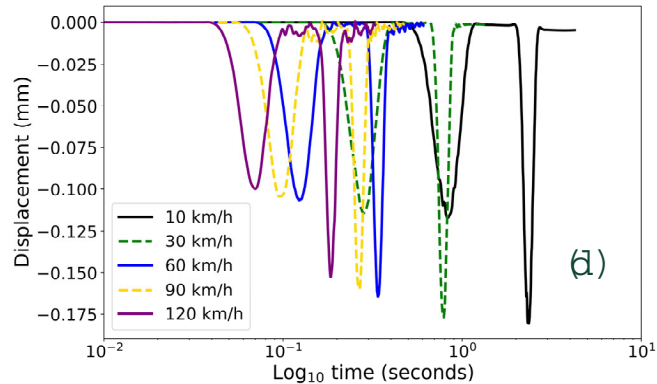
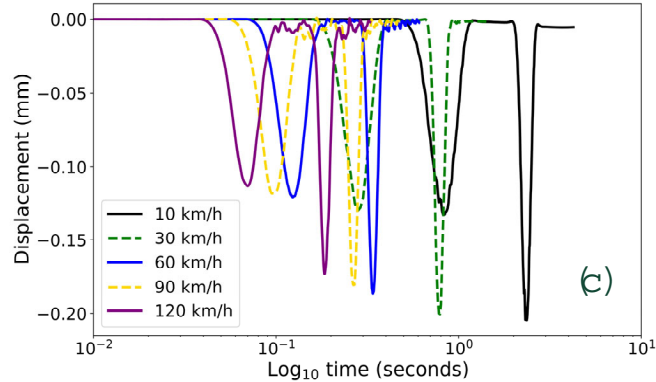
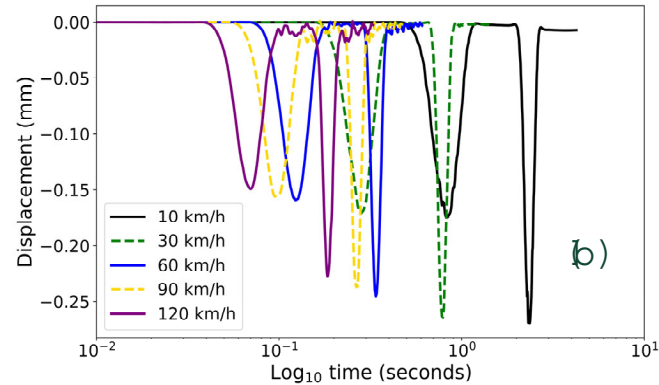
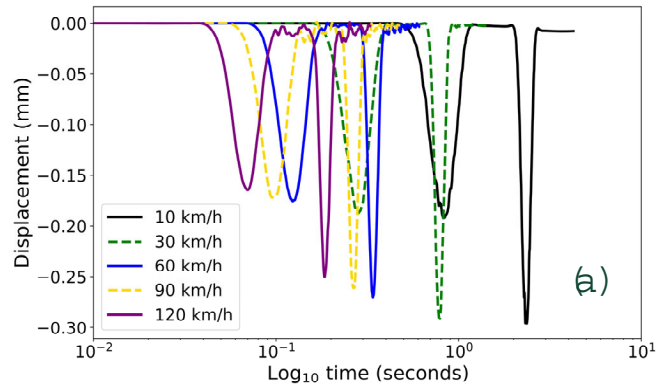
Note: No. = number of Prony-Series; τ = relaxation time; and G = dimensionless relaxation modulus.

Finite Element Modeling



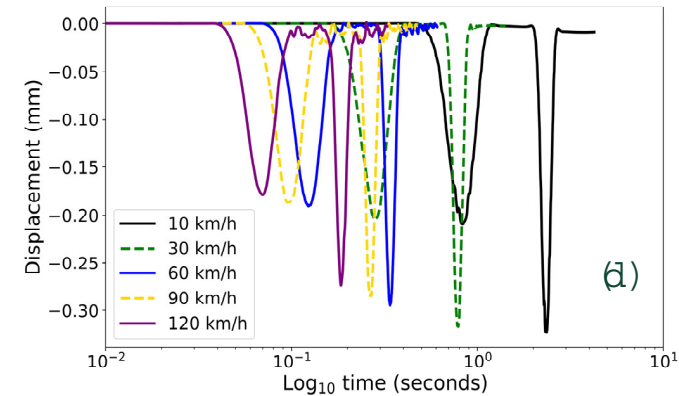
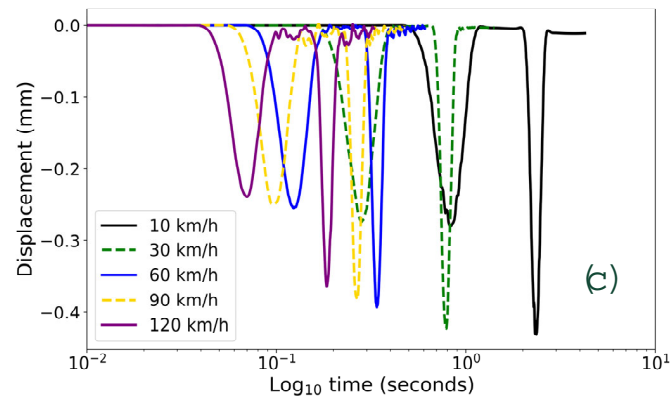
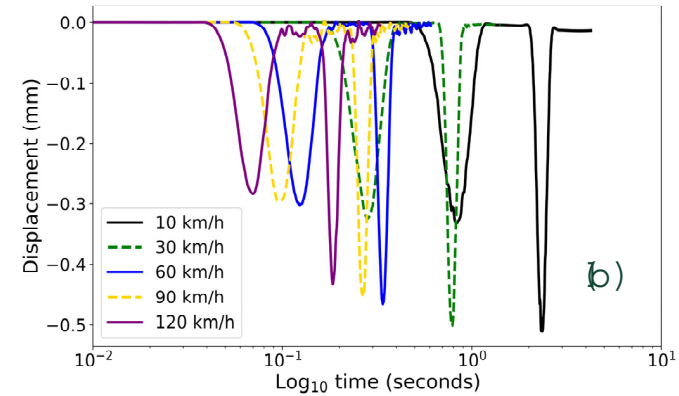
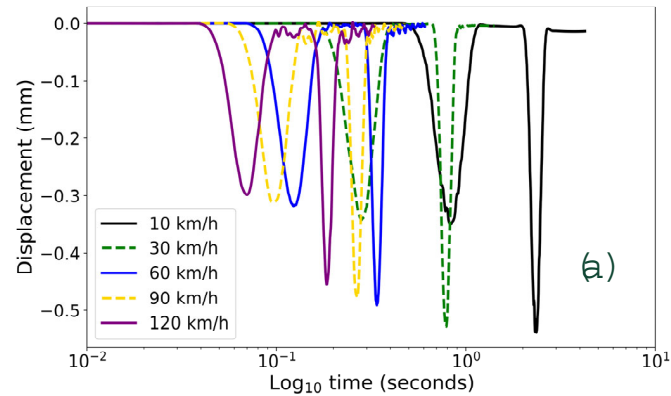


Ambient Temperature



Crown displacement versus time response under a moving truck with different truck speeds for (a) very flexible pipe (10 kN/m) (b) flexible pipe (30 kN/m) (c) semi-rigid pipe (100 kN/m) (d) rigid pipe (1000 kN/m)

Elevated Temperature



Crown displacement versus time response at elevated temperatures under a moving truck with different truck speeds for (a) very flexible pipe (10 KN/m), (b) flexible pipe (30 KN/m), (c) semi-rigid pipe (100 KN/m), (d) rigid pipe (1000 KN/m).

Thank You !